

NAVAL FACILITIES ENGINEERING SERVICE CENTER Port Hueneme, California 93043-4301

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### **BIOPILE DESIGN AND CONSTRUCTION MANUAL**

by

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and

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#### **BIOPILE DESIGN AND CONSTRUCTION MANUAL**

Prepared for

Naval Facilities Engineering Service Center Port Hueneme, California

17 June 1996

Prepared by

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FIGURES		iv		
TABLES		v		
ABSTRACT		vii		
Section 1.0: INT	RODUCTION	. 1		
1.1	Background and Objectives	. 1		
1.2	<ul> <li>1.1 Dackground and Objectives</li> <li>1.2 Overview</li> <li>1.3 Biopile Technology Screening and Selection</li> </ul>			
1.3	Biopile Technology Screening and Selection	. 2		
1.4	Scope of the Manual	. 4		
Section 2.0: TEC	HNOLOGY OVERVIEW	5		
2.1				
2.2	Base Preparation	6		
	Leachate Collection	7		
2.4	Aeration	 7		
2.5	Moisture Addition	 7		
2.6	Nutrient Addition	. , 8		
2.7	Microbial Amendment	ġ		
	General Construction	ġ		
2.0		. •		
Section 3.0: REC	ULATORY ISSUES AND PERMITTING STRATEGY	13		
	Regulatory Issues			
3.2	Permitting Strategy	13		
Section 4.0: PRO	FILE OF EXISTING CONTAMINATED SOILS	16		
4.1	Contaminant Sources and Distribution	16		
4.2	Contaminant Properties	17		
4.3				
4.4	Influence of Soil Properties on Biopile Treatment	21		
	4.4.1 Moisture Content	21		
	4.4.2 Soil pH	21		
	4.4.3 Nutrient Supply			
	4.4.4 Metal Content	22		
Section 5.0. DPL	DESIGN ACTIVITIES	93		
	Treatability Studies	20 27		
	<ul> <li>5.1 Treatability Studies</li></ul>			
	<ul><li>5.2 Decision Between Temporary and Permanent Site</li><li>5.3 Biopile Site Selection</li></ul>			
5.4				
	Cost Analysis	ະມ 31		
J.J		51		

### CONTENTS (Continued)

Section 6.0: BIOP	PILE CONSTRUCTION	34
6.1	Temporary Biopile Construction	
	6.1.1 Site Preparation	
	6.1.2 Biopile Base Construction	
	6.1.3 Aeration System	
	6.1.4 Nutrient Addition	
	6.1.5 Moisture Addition	
	<ul><li>6.1.6 Leachate Collection System</li><li>6.1.7 Biopile Formation</li></ul>	
	6.1.7 Biopile Formation 6.1.7.1 Soil Preparation	
	6 1 7 9 Pile Formation	
	6.1.7.2 Pile Formation 6.1.7.3 Installing Soil Gas Monitoring Points	
	6.1.7.4 Time-Zero Sampling	
	6.1.7.5 Cover Installation	
6.2	Permanent Biopile Design	
	6.2.1 Concrete Biopile Pad	54
	6.2.2 Aeration System	55
	6.2.3 Leachate Collection System	58
	6.2.4 Automated Irrigation System	
	6.2.5 Nutrient Addition	
0.0	6.2.6 Soil Processing	
6.3	Sampling and Analysis 6.3.1 Soil Sampling Methods	
	<ul><li>6.3.1 Soil Sampling Methods</li><li>6.3.2 Soil Gas Sampling and Soil Temperature Data Collection</li></ul>	
	6.3.2.1 Tedlar <sup>TM</sup> Sampling Bag Method	
	6.3.2.2 Direct Soil Gas Sampling Method         6.3.3 Leachate Sampling Method	02 62
Section 7.0: REFE	ERENCES	
APPENDIX A:	GLOSSARY OF TERMS USED TO DESCRIBE BIOPILE DESIGN AND	
	IMPLEMENTATION	
APPENDIX B:	OVERVIEW OF APPLICABLE FEDERAL ENVIRONMENTAL	70
	REGULATIONS	
APPENDIX C:	STATE ENVIRONMENTAL REGULATORY AGENCIES	78
AITENDIA C.	STATE ENVIRONMENTAL REGULATORY AGENCIES	
APPENDIX D:	EPA REGIONAL OFFICES	82
APPENDIX E:	CHEMICAL AND PHYSICAL PROPERTIES OF PETROLEUM	
	HYDROCARBONS	
APPENDIX F:	COMPANIES DOING TREATABILITY STUDIES	
APPENDIX G:	METHODS OF DETERMINING FIELD CAPACITY	
A DDENIDIV II		0.0
APPENDIX H:	BIOPILE COST ESTIMATOR <sup>©</sup> COST SHEETS FOR SAMPLE CASES	
APPENDIX I:	RESPIRATION TEST DATA CALCULATION AND WORKSHEET	117
	MOTING TOTAL OF DATA ON COLOCIATION AND WOMSTILLT	
APPENDIX J:	EXAMPLE DESIGN CALCULATIONS	121
	•••••••••••••••••••••••••••••••••••••••	

### CONTENTS (Continued)

#### FIGURES

Figure 1.	Biopile Technology Selection Decision Tree.	
Figure 2.	Leachate Collection System Incorporated with the Aeration System for a Permanent	
U	Biopile Design	6
Figure 3.	Biopile Design Soil Processing Equipment Train	10
Figure 4.	Layout of a Typical Biopile Aeration System.	11
Figure 5.	Permitting Process.	13
Figure 6.	Conceptual Model of the Distribution of Contaminants.	17
Figure 7.	Illustration of the Types of Organic Compounds in Petroleum	18
Figure 8.	Relationship Between Contaminant Vapor Pressure and Aerobic Biodegradability.	20
Figure 9.	Predesign Activities Checklist.	
Figure 10.	Plan View of a Biopile Site Layout	30
Figure 11.	Plan View of a Site with MultipleBiopile Modules	31
Figure 12.	Biopile Construction Checklist	35
Figure 13.	Example Parts List	36
Figure 14.	Site Preparation Checklist	37
Figure 15.	Construction Design of a Typical Temporary Biopile	38
Figure 16.	Construction Design Detail of a TemporaryBiopile.	39
Figure 17.	Aeration System for a TemporaryBiopile Configured to Operate in the Extraction Mode	42
Figure 18.	Biopile Nutrient Addition Worksheet	
Figure 19.	Typical Dripline Irrigation System.	
Figure 20.	Typical Monitoring Point Construction.	
Figure 21.	Suggested Monitoring Point Emplacement Technique.	51
Figure 22.	Suggested Pattern for Tying DownBiopile Cover with Nylon Rope	53
Figure 23.	General Concrete Biopile Schematic	
Eiguna 94	Joint Construction of the Constants Dad Side Soming on the London Entropy Daint	50
Figure 24.	Joint Construction of the Concrete Pad Side Serving as the Loader Entrance Point.	56
Figure 25.	Permanent Biopile Aeration System Design: (a) aeration channel; (b)aeration pipe	r ~
Et	construction at berm intersection	
Figure 26.	Schematic of an Installed Irrigation System.	59
Figure 27.	Typical Soil Processing Flowchart61	

#### TABLES

Table 1.	Testing Required for Biopile Feasibility	25
Table 2.	Biopile Site Selection Guidelines	29
Table 3.	Summary of BPCE Cost Analyses for a Biopile Construction and Operation	32
Table 4.	Monitoring Point Parts List	48
Table 5.	Types of Nutrient Sources	60
Table 6.	Summary of Analytical Methods for Biopile Design	62

#### ABSTRACT

This manual was created to supportbiopile work being conducted at U.S. Navy and Marine Corps facilities. This document details the selection procedures and design and construction steps for implemening the biopile technology, which is a method for ex situ treatment of soils comminated with petroleum hydrocarbons. The manual provides a general overview of thebiopile technology, followed by detailed descriptions of selection criteria, regulatory issues, design parameters, and construction procedures. Depending on factors such as volume of soil to be treated, material handling, and the available area to construct thebiopile, one may choose to construct a single cell or a number of similar cells (modular approach). For the purpose of consistency and to facilitate numerous calculations, a baseline 500-yd<sup>3</sup> (382-m<sup>3</sup>) treatment cell is considered in this manual. Two designs are presented for the baseline pile size of 500 yd<sup>3</sup> (382 m<sup>3</sup>): one for a temporary system, and one for a permanent system.

The scope of this document is to present the design and construction procedures for **b**iopile. A companion document titled *Biopile Operations and Maintenance Manual* (TM-2190-ENV) provides detailed procedures for biopile system operations (e.g., sampling) and maintenance, and information to decide when to terminate the system operation. The document is composed of a main body and a series of appendices containing relevant technical and support details.

#### **BIOPILE DESIGN AND CONSTRUCTION MANUAL**

#### Section 1.0: INTRODUCTION

**1.1 Background and Objectives** . A large number of U.S. Department of Defense (DoD) sites reportedly have petroleum- and fuel-contaminated soils and groundwater as a result of leaking underground storage tanks (USTs) and pipelines or other accidental releases. With so many sites requiring remediation at relatively high costs, the Naval Facilities Engineering Service Center (NFESC) has been developing and demonstrating more effective and less costly remedial alternatives. The NFESC has successfully demonstrated the applicability obiopiles to reduce the concentration of petroleum constituents in excavated soils through the use of aerobibiodegradation. The purpose of this manual is to provide design features for abiopile; a companion document titled*Biopile Opera tions and Maintenance Manual* (NFESC, 1996, TM-2190-ENV) presents operations and maintenance procedures for **b**iopile.

**1.2 Overview.** Biopile technology involves forming petroleum-contaminated soils into piles or cells above ground and stimulating aerobic microbial activity within the soils through aeration. Micrbial activity can be enhanced by adding moisture and nutrients such as nitrogen and phosphorus. The aerobic microbial activity degrades the petroleum-based constituents adsorbed to soil particles, thus reducing the concentrations of these contaminants. Biopiles typically are constructed on an impermable base to reduce the potential migration ofleachate to the subsurface environment. A perforated piping network installed above the base is connected to a blower that facilitates the aeration of the pile. In some cases, aleachate collection system is constructed, especially if a moisture addition system is being considered for the pile. The piles generally are covered with an impermeable membrane to prevent the release of contaminants and/or contaminated soil to the environment and to protect the soil from wind and precipitation. Biopiles operate effectively in temperate climates but can be operated in colder climates by introducing warm air through the aeration process.

The advantages of the biopile technology include the following:

- The contaminants are destroyed, making this a toxicity reduction process that is preferred by the regulators.
- Biopile systems are relatively easy to design and construct.
- Remediation can be completed in a relatively short time (3 to 6 months). Futurcontainment of the treated soil is not required.
- Biopiles offer a cost-competitive technology compared to thermatlesorption, which is another commonly used aboveground treatment technology.
- Biopiles can be cost-competitive with landfilling and are preferred over landfilling.
- Biopile technology is effective on organic contaminants that are difficult todesorb.
- Biopiles can be engineered to be potentially effective for any combination of site conditions and petroleum products.

The biopile technology has the following limitations:

- Biopiles may not be effective for high contaminant concentrations (>50,000pm total petroleum hydrocarbons). However, such levels are not common in UST sites. During excavation, the peak contaminant levels are reduced, because highly contaminated soil becomes mixed with surrounding soil that is less contaminated.
- The presence of significant heavy metal concentrations (>2,500ppm) may inhibit microbial growth.

**1.3 Biopile Technology Screening and Selection**. The use of a biopile to remediate petroleum hydrocarbon contamination from soil is generally applicable under the following conditions:

- Soil is contaminated primarily with petroleum hydrocarbons.
- Chlorinated or recalcitrant organic compounds are present in negligible amounts.
- Toxic metal concentration is below 2,500 mg/kg soil.
- The total soil volume to be treated is greater than 250 yd.
- There are multiple sources of TPH-contaminated soil to be treated that can total more than 250 yd<sup>3</sup>.
- The TPH treatment target levels usually range from 500 to 1,000 mg/kg of soil. Actual target levels should be negotiated with the appropriate regulatory agency on a site-by-site basis.

The bulleted conditions listed above serve as guidelines and are not hard and fast rules. Site-specific conditions may dictate whenbiopile treatment may be a successful alternative. However, under normal circumstances, the above-cited conditions should be viewed as the primary qualifying guidelines in evaluating whether or not to pursue the use of abiopile system. In any event, the project manager should conduct economic and technological assessments. This document has been designed to give the project manager the required information to make a sound technological evaluation of the applicability of abiopile for a specific soil remediation need. The project manager can use the decision tree shown in Figure 1 to properly screen the feasibility of **b**iopile system. This decision tree is predicated on the above-listed bulleted conditions.

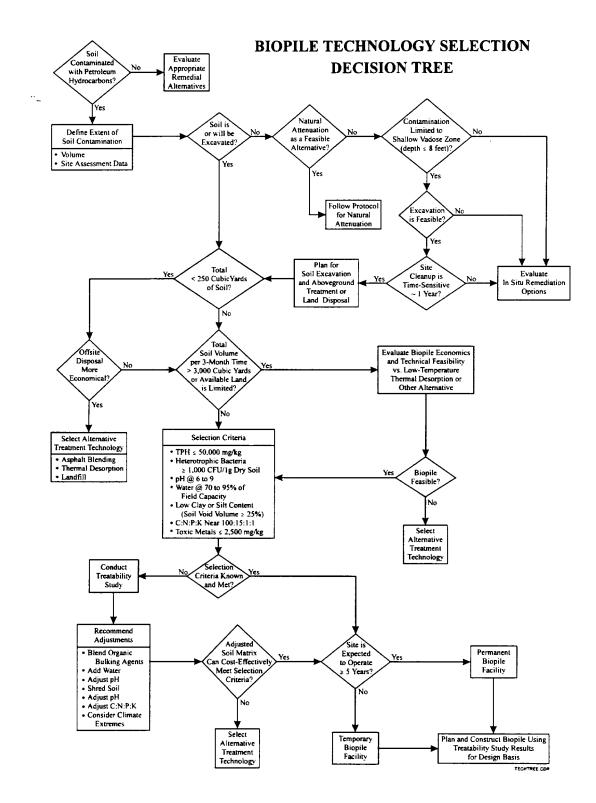


Figure 1. Biopile Technology Selection Decision Tree.

**1.4 Scope of the Man ual.** This manual is intended to provide technical guidance on the design, operation, and maintenance of biopiles to remediate soils contaminated with petroleum-based organic contaminants. The design and construction manual is followed by a companion document that includes procedures for operation and maintenance. The design and construction manual focuses on engineering-related considertions for biopiles. It does not provide detailed design specifications and civil engineering construction details (e.g., design and construction of concrete pads). Such information should be site-specific and the reader should seek local engineering guidance.

The design and construction manual consists of seven sections. Section 2.0 provides background on the biopile concept from a literature review and case histories. Section 3.0 presents the regulatory issues and permitting strategy. Section 4.0 describes how to profile soils to determine their applicability to remediation ibiopiles. Section 5.0 presents preliminary design information, and Section 6.0 provides the design and construction specifications for a baseline biopile volume of 500 yd<sup>a</sup> (382 m<sup>3</sup>). Section 7.0 lists references cited in text. Appendices A through J provide supplementary information to assist the designer.

#### Section 2.0: TECHNOLOGY OVERVIEW

This section introduces the technical basis forbiopile design by describing prior applications ofbiopiles to the treatment of contaminated soils. Information about past experience withbiopiles is organized into eight groupings based on major functional elements ofbiopile systems. These elements are as follows:

- site preparation
- base preparation
- leachate collection
- aeration
- moisture addition
- nutrient addition
- microbial amendment
- general construction.

Biopile treatment is a controlled biological process where biodegradable contaminants are converted to their basic mineral constituents (water and carbon dioxide) under aerobic conditions. Soil is excavated, prepared, formed into a biopile, and aerated to promotebiodegradation. In most cases, thebiodegradation is achieved by indigenous microorganisms. Maximum degradation efficiency is achieved by maintaining the moisture content, pH, aeration, temperature, and carbon-to-nitrogen ratio (DOD, 1994). Terms used to describ**b**iopile design are defined in Appendix A.

**2.1 Site Preparation .** The selected site must provide adequate space, infrastructure, and support services for the biopile. A flat area, free of obstructions, is needed for soil handling and pile construction. The site serves as the subgrade for the base that carries the weight of thebiopile and associated soil handling and preparation equipment. Roads and bridges leading to the site must be capable of carrying legal-weight, legal-length trucks with a gross vehicle weight of 40 tons (36 tonnes) and an approximate maximum payload of 24 tons (22 onnes). Space is required for stockpiling, mixing, and preparing soil as well as for thebiopile. Electrical service will be needed to operate equip ment such as blowers, pumps, and instruments. A 480-V, 3-phase electrical supply is desirable if the blower size is over 2 hp, and is strongly recommended when the blower size is over 4hp. In addition, 110-V single-phase service is desirable to operate small pumps and AC-powered instruments.

#### **2.2 Base Preparation**. The base of the biopile serves three main functions:

- It provides a stable foundation to support thebiopile and associated soil handling operations.
- It provides a barrier against potential migration of contaminants into the underlying soil.
- It provides a 1% grade to avoid pooling ofleachate at the base of the biopile.

The biopile base may be newly laid or the pile may be built on an existing foundation such as a parking lot or a storage yard. A new base for a permanent facility may be constructed from Portland cement concrete or bituminous paving. To lower the potential for contaminant migration, either type of base normally would be covered with an impervious liner (Kamnikar, 1992) and (Brown andCartwright, 1990). For a permanentbiopile, the impervious liner may be placed beneath the compacted soil foundation of the concrete pad. The base for a temporarybiopile may be constructed using a compacted soil or clay and should be covered by a replaceable impervious liner (Brown and Cartwright, 1990). Using an existing paved area for thebiopile reduces construction costs but requires special provisions for underdrains and sloping to collectleachate.

**2.3 Leachate Collection**. For a permanentbiopile site, a leachate collection system should be installed. The leachate collection system usually includes a containmentberm or structure around the pile, perforated pipe at low points in the fill, a leachate collection pump connected to the drain piping, and deachate collection tank.

For smaller biopiles, the leachate collection system can be incorporated into the vapor extraction system. Experience has shown that most water will flow to the aeration pipes. Very little or not achate would flow to a low-point leachate collection sump in a covered pile system. Using this type of design, a liquid-knockout tank is installed in the vapor extraction manifold ahead of the blower. A small vacuum pump is then periodically used to pump the water from the knockout tank to theleachate collection tank. Figure 2 presents a schematic of aleachate collection system.

**2.4** Aeration. The biopile must be adequately aerated to support efficient degradation of contaminants by microorganisms. Of all the metabolic factors, oxygen is the most important, so efficient aeration is essential thiopile

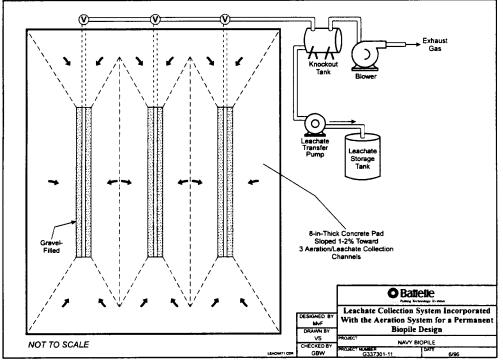


Figure 2. Leachate Collection System Incorporated with an Aeration System for a Permanent Biopile Design.

success. Both active and passive air supply systems have been used successfully (Brown an Cartwright, 1990; Kamnikar, 1992).

The simplest method of pile aeration is a passive system. Drain tiles, perforated tubing, or slotted pipes are placed at various heights throughout the pile. The tiles or pipes are long enough so their ends stick out of the pile, allowing air transfer and venting due to natural currents Kamnikar, 1992). The passive method reduces capital and operating costs because no blower is required, and reduces the potential for drying the pile because the airflow rate is low.

Despite the lower cost for passive aeration systems, active aeration is preferred, because it gives more complete and more controllable airflow that speedsbioremediation in the pile. Two active aeration configurations have been used: air injection and air extraction. Both configurations have perforated pipes located in the pile that are connected to a blower to push air into the pile (injection) or to pull air through and out of the pile (extraction). Typically, airflow rates through the pile are just great enough to keep the soil above oxygen-limiting conditions. Such flowrates prevent excess volatilization of hydrocarbons, thus reducing the vapor emissions from the pile. One method for determining the required airflowrate is presented inLeeson and Hinchee (1995). Although this method was developed for in situ treatment of hydrocarbons, it can be used fobiopile facilities. The PILEFLOW computer modeling method for biopiles (NFESC, 1996c) is a more relevant airflow modeling method. The PILEFLOW model is

#### described in AppendixK.

The contaminant volatilization rate also is dependent on the type of contaminant present in theippile. Operating in the extraction configuration often is preferred when volatilization of organic compounds is a concern (Hayes et al., 1995). In the extraction configuration, emissions from the pile can be collected and controlled. Extracted air from the blower outlet is passed through a treatment system to destroy contaminant vapors. Granular activated carbon (GAC) historically has been used atbiopile sites for the treatment of discharge vapors.

When the TPH contamination is a heavier fuel (diesel or heavier), off-gas treatment may not be necessary. The TPH concentration in thebiopile exhaust air will rapidly decline as the minor, lighter hydrocarbon fraction is depleted. Chaudhry (1996), for example, reported a decline of TPH irbiopile exhaust air from an initial 733 ppmv to below 2 ppmv within 3 weeks of system startup. Efforts should be made to negotiate with regulators not to require biopile off-gas treatment in cases where the primary contaminant is a heavier hydrocarbon. In some cases, initial vapor treatment followed by no vapor treatment may be an appropriate option. Vapor treatment could be stopped once TPH concentrations in thebiopile exhaust have decreased below a negotiated level.

**2.5 Moisture Addition .** Water must be available in the biopile, but the amount must not be excessive. Microorganisms require moisture to transport nutrients, to carry out metabolic processes, and to maintain cell structure. However, excessive moisture is undesirable because (a) when water occupies a high fraction of the pore space in the soil, the air permeability declines, reducing aeration efficiency; and (b) excess moisture will increase leaching of contaminants and nutrients from the pile.

The moisture content and moisture retention characteristics of soils to be treated in *b*iopile can readily be adjusted during the initial preparation of the soil for thebiopile. At some sites, little or no initial water addition will be needed. If the moisture content is too high, dry bulking agents can be mixed with the soil. The bulking agent can be selected to increase or decrease moisture retention. Also, some data indicate that surfactant addition may improve surface wetting (U.S. EPA, 1990, EPA/540/2-90/002).

The moisture content of the biopile may change as the remediation proceeds. Air nomally will enter the biopile at less than 100% relative humidity. The air will tend to remove moisture as it moves through the biopile and become saturated with water, thus reducing the moisture content. However, at the same time, the indegradation process is converting hydrocarbons to CQ and  $H_2O$ , thus renewing the moisture content to some degree. Approximately 1.51b (0.68 kg) of  $H_2O$  is produced per 1 lb (0.45 kg) of TPH degraded. Depending on the site conditions, it may be necessary to add moisture duringbiopile operation. However, dry ambient air conditions and low initial hydrocarbon content together or individually tend to increase the need for moisture addition. Moisture control practices are easily implemented as long as the biopile design and installation provide forleachate collection and control.

Unless the feed air is dry, the aeration rate is excessive, or the soil organic content is low, **a** initial adjustment of moisture content usually is sufficient to eliminate the need for water addion during operation. Under normal conditions, a coveredbiopile system should be expected to lose 1 to 2% of moisture over a 3- to 4-month operating period. Onebiopile study used wetted wood chips and manure mixed with the soil as additional moisture sources instead of providing moisture additon during pile operation (Kamnikar, 1992). Generally, the moisture content is kept between 40and 85% of field capacity throughout the remediation process (U.S. EPA, 1995). Although the U.S. Environmental Protection Agency (U.S. EPA) recommends that the moisture content be kept between 40 and 85% of field capacity, it is suggested that the soil be between 70 and 90% of field capacity if water is applied only during biopile construction, because it is assumed that some drying (1 to 2weight %) will take place during the pile construction and operation.

**2.6** Nutrient Addition. Biopiles work to degrade contaminants by means of the microrganisms in the pile that use the contaminants as a source of carbon and energy. The organisms need a supply of carbon to buildiomass. The contaminants and natural organic compounds in the soil typically provide an adequate amount of carbon, but the availability of other essential nutrients such as nitrogen, phosphorus, or potassium may be insufficient compared to the quantity of carbon. Typically, the C:N:P ratio is brought to within the range of 100:10:1 to 100:10:0.5 (U.S. EPA, 1995).

In general, the soil should be amended with nutrients prior tobiopile construction (Brown and Cartwright, 1990). The nutrients may be either dissolved in water and sprayed onto the soil prior to construction of the pile or applied in granulated form and mixed with the soil while the pile is being constructed.

Nutrients may be added during operation. The nutrient addition is combined with a moisture addition system. When the pile begins to dry out, a dilute solution of nutrients in water is applied to the top of the pile using sprays or drip irrigation systems. The nutrient solution then percolates down through the pile (Brown an Cartwright, 1990). Nitrogen permeates through to the bottom of the pile; however, phosphorus generally travels down 1 to 2 ft due to chemical reactions with the soil.

Biopile systems have been designed to irrigate the pile with water recovered in the achate collection system. In most cases, the water is pumped from the achate collection tank and is recycled to the pile using a drip-type irrigation line (USACE, 1995).

2.7 Microbial Amendment . Some biopile designs have included the addition of microbes along with the nutrients. The microbial amendment is added to the nutrient solution and is sprayed onto the soil in preparation for biopile construction. The organisms are naturally occurring but are claimed to be specially cultured to optimize hydrocarbon degradation (Shaw et al., 1995). Microbial amendments increase the overall cost and have not been clearly demonstrated to improve the degradation of petroleum hydrocarbons. Amendments such as white-rot fungi may be necessary to degrade recalcitrant compounds, but mosbiopile users reject the addition of exogenous microbes. Many studies indicate that indigenous microorganisms are capable of successfully degrading petroleum hydrocarbon contaminants (Hinchee et al., 1992).

**2.8 General Construction**. Most soils will require the addition of water (see Section2.5) and nutrients (see Section 2.6) and will then be ready to be treated. In some cases, the nutrient and water contents of the soil to be treated are adequate and the grain size is coarse enough to provide good air permeability without adding anything to the soil.

In soils with a high clay content, soil shredding and/or blending with a bulking agent may be needed to improve the soil structure and porosity. Typical bulking agents are wood chips or sand (Brown an**C**artwright, 1990). Prior to any soil shredding, a screening step should occur to remove rocks and debris. The typical treatment train to prepare soils requiring the addition of bulking agents includes the use of a set of parallel metal bars (grizzly) for bulk separation followed by soil shredding and then addition of water and nutrients. Figure 3 gives a schematic of such a soil preparation process. The equipment and procedures to mix soil range from basic to complex. In the simplest system, thin layers of soil and bulking agent are interspersed, wetted, and then lifted and dropped using a front-end loader (Kamnikar, 1992). If a higher throughput or better control of volatile emissions is required, a faster mixer with a better seal, such as a pug mill, may be used. Crushing and mixing of the soil may be desirable to increase the contaminant homogeneity and improve the soil permeability and may eliminate the need to add a bulking agent (Eiermann and Bolliger, 1995).

The soil, as received (most cases) or as processed (in cases where soil shredding or bulking agents are required), is mixed with the appropriate amounts nutrients and water as the soil lifts are added to the propriate foundation. The nutrients may be added as part of a metered water-spray solution or may be added dry in measured quantities as each additional bucket of soil from the is unloaded on the propriate. The amount of water and nutrients to add per bucket load is a function of the size of the

bucket. The biopile ingredients must be blended in the correct ratios for optimunbiodegradation of the contaminants before or during pile construction (see Section 2.6). The pile is then arranged to allow efficient aeration while minimizing contaminant and odor release and contr**d**ing the internal temperature.

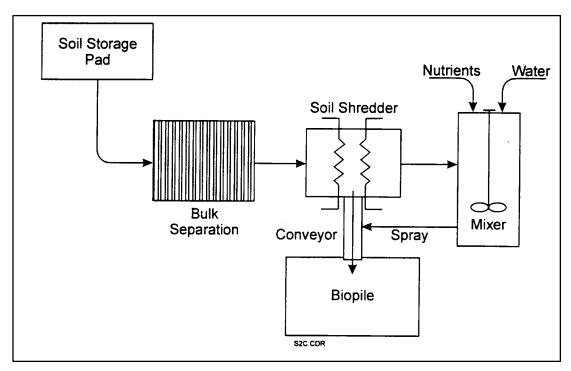


Figure 3. Soil Processing Equipment Train.

The construction details of biopiles are flexible. Effective biopiles have been built in a wide variety of sizes and shapes. Historically, biopile dimensions have been constrained more by space availability and logistics than by size-based performance limitations. Although a wide range of sizes and shapes have been used biopile construction complexity increases significantly when the pile dimensions exceed the reach of a front-end loader. Thus, biopile dimensions usually do not exceed a height of &t (2.4 m). There are no general length or width restrictions, but the front-end loader must avoid driving over previous lifts.

The task of evenly aerating the pile influences the size and shape of theoiopile. Tall piles (>10 ft [>3 m]) generally require more than one level of aeration pipes, thus complicating the co**s**truction process. When installing multilevel aeration systems, the aeration pipes frequently are placed on top of each layer of soil and covered by the next layer so that the pipes are located at varous heights within the pile. Experience, however, indicates that a single set of aeration pipes located at the bottom of the pile is adequate for piles up to about 8 to 10 ft (2.4 to 3.0 m) high (Figure 4). Aeration pipes must be arranged so that they are not crushed by earth-moving equipment. To avoid com pacting the biopile, earth-moving equipment should never be allowed to travel over the soil in the pile.

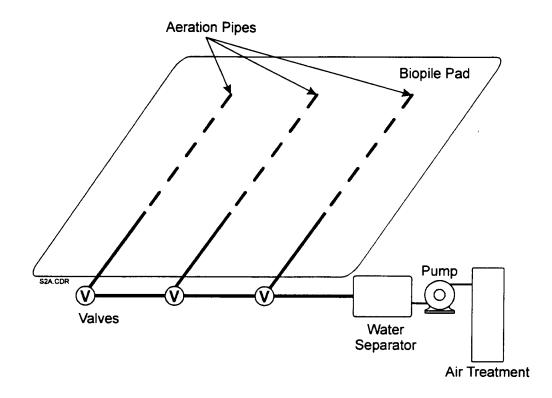


Figure 4. Layout of a TypicalBiopile Aeration System.

Biopile construction typically includes installation of instruments within the pile to monitor critical parameters. The most essential monitoring provisions are tubes for sampling gas in the pile and thermocouples to measure temperature (Kamnikar, 1992).

After the pile is formed, it should be covered. The cover serves to protect the biodegrading soil from the elements to:

- retain moisture
- retain heat
- prevent excessive, sudden water addition from rain
- prevent wind from blowing dust from the pile
- prevent cementation of upper soil layer from wetting and drying.

Plastic sheeting material held down by old tires, sand bags, or weighted netting is often selected as a low-cost, effective approach for biopile protection (Brown andCartwright, 1990). A successful method of securing the cover, developed by the Navy, uses nylon rope to hold down the cover. The rope is held in place by passing it through eye-bolts screwed into the biopile berm. In a few cases, an existing building has been used, or an inexpensive structure such as a pole barn or sheet metal building has been built in lieu of a cover.

Proper construction of the biopiles is important to avoid excessive internal temperatures (Shaw et al., 1995). Biological degradation of contaminants releases heat in the same way combustion would, but at a lower rate. The heat release increases the biopile temperature during operation. Some temperature rise is desirable to enhance the microbial degradation rate, but an excessive temperature increase is undesirable because bioactivity declines after the optimum temperature is reached. The typical target temperature falls in the range of 68 to 10% (20 to 40°C) (Lei et

#### al., 1994).

The biopile construction design should include plans for demobilization and decontamiation. When soil treatment is complete, the equipment should be cleaned and removed, and the site restored.

#### Section 3.0: REGULATORY ISSUES AND PERMITTING STRATEGY

Before a remedial action is taken, regulatory research must be performed to determine which regulations will govern the site. The project officer must be familiar with the site geography and topography as well as the type of contamination, the contaminated medium, and the remediation method to be used to know which regulations may apply. Additionally, an understanding of potentially relevant environmental regulations will be useful in mapping out the permitting approach for the specific site.

**3.1 Regulatory Issues**. Environmental regulations are categorized as chemical-specific, location-specific, and action-specific regulations. Each type may be issued at the federal, state, or local level. Examples of chemical-specific regulations include the federal Clean Air Act and Clean Water Act which set numerical limits on the emissions and discharges of specific substances. Some chemical-specific regulations also specify target cleanup limits. Location-specific regulations include laws protecting site-specific resources such as endangered wildlife, wetlands, and wilderness areas. Action-specific regulations apply to specific activities or technologies and include monitoring requirements, effluent and leachate discharge limits from specific processes, and worker health and safety requirements. The water and air discharges frombiopiles are required to meet the permitting standards of the local jurisdiction. Appendix B describes the major federal environmental policy acts and regulations and provides a tabular listing of various chemical-, location-, and action-specific regulations.

Time must be budgeted within the remediation plan to allow for the regulatory research and permitting processes. It may take weeks or months after identifying the lead regulatory agency to obtain the permits required to begin a bioremediation action. Every stage of the cleanup effort from planning to shutdown must be coordinated with the appropriate regulators.

**3.2 Permitting Strategy**. This section describes a general strategy for working with environmental regulatory agencies and obtaining the proper permits to implemenbioremediation projects such asbiopiles. Figure 5 illustrates the elements of the permitting process.

It is important to have the potential regulatory agencies involved in the permitting process while the biopile is still in the planning stages. The project officer should use the following steps as a guideline to execute the permitting process:

- Step 1. Establish the contamination type, contamination level, and amount of soil to beremediated.
- Step 2. Decide on the treatment alternative(s) using the decision tree in Section 1.
- Step 3. Select a tentative treatment location.
- Step 4. Prepare a tentative work plan.
- Step 5. Identify and contact applicable regulatory agencies.
- Step 6 Through meetings and correspondence, determine the lead regulatory agency.

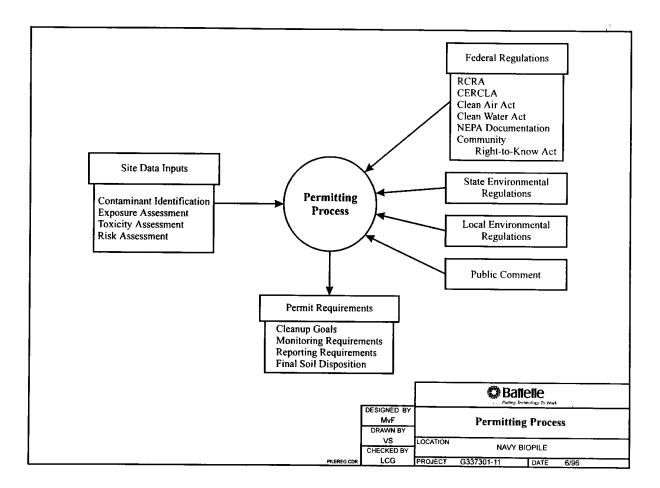


Figure 5. Permitting Process.

Step 7.	Negotiate the target cleanup levels and final soil disposition with the lead
	agency. Obtain a written memo from the regulatory agency confirming the
	agreed-upon cleanup levels, and all documentation and permits that will be
	required for the project.
Step 8.	Work through the lead agency to obtain the proper permits.

- Step 9. Coordinate the site preparation and soil remediation plan with the base environmental and facilities departments. Be sure to include plans for final soil transportation, disposition, and sitecloseout.
- Step 10. Inform other agencies as necessary of planned remediation activities.

The U.S. EPA and state environmental protection agencies, as well as local water and air quality boards and the landfill authority, may simultaneously regulate remediation efforts. When regulations of different governmental levels overlap, the most stringent regulation applies. Rather than coordinating a remediation plan through every one of these agencies, the jurisdiction of each agency should be compared to the applicable environmental risk of the contamination and to the site. The most relevant agency should be identified as the lead agency. Once the lead agency is known, relevant regulatory matters can be handled with it directly, while keeping the other regulatory organizations informed of the situation through mail or meetings. Before initiating communications with regulators, thebiopile project officer should prepare a tentative work plan outlining the nature and extent of the contamination, proposed biopile location, preliminary remediation timetable, and proposed final soil disposition. A good work plan will facilitate the regulatory evaluation of the proposed remediation effort and may reduce the overall time required to complete the permitting process.

Part of the regulatory process involves negotiating with the lead agency to set the target cleanup levels to be achieved through the proposed remediation. Prior tobiopile construction, target cleanup levels should be negotiated with the lead agency based on the type of contaminant, extent of contamination, and limits of the technology. Sometimes the chosen cleanup levels are set based on public drinking water standards or Resource Conservation and Recovery Act (RCRA) Land Disposal Restriction (LDR) requirements. However, higher concentrations may be acceptable if risk-based analysis shows that the concentration is protective of human health and the environment.

To be acceptable, remediation of thebiopile must be able to reach the required cleanup limits. For a biopile, the target cleanup level for total petroleum hydrocarbon (TPH) contamination generally is in the range of 500 to 1,000 ppm for a 3-month treatment cycle. Levels of 100 to 500ppm may be achievable for longer treatment periods (4 to 6 months). The actual target cleanup level is dependent on the initial TPH contamination level, soil type, desired final soil disposition, and the applicable regulatory agency's specifications. When the target cleanup level has been established, the biopile project officer should request a written letter from the agency point of contact specifying the agreed-upon target cleanup level and the corresponding method of final soil disposition.

After establishing the target cleanup level, the project officer continues to coordinate and complete the permitting process. Once all applicable permits have been obtained, the work plan can be finalized by coordinating the biopile design and site requirements with the base environmental and facilities offices. Plans for final soil transport and disposition in accordance with the site permits must be in place prior to the start of construction. Copies of the final work plan may be sent to the appropriate regulators and base officials, as necessary.

Appendix C contains a list of the hazardous waste management contacts for each state. Appendix D contains a list of the addresses and phone numbers of the U.S. EPA Regional Offices throughout the country.

#### Section 4.0: PROFILE OF EXISTING CONTAMINATED SOILS

The media of concern for this design and construction mamal are soils contaminated with petroleum hydrocarbons. These materials are hydrocarbon products with minor additives specific to each product's application and consist mainly of straight-chain and cyclic organic compounds.Biopile remediation is most likely to be applicable when petroleum hydrocarbon contamination sources are limited to soil depths shallower than 8 ft. In general, in situ treatment is applied at deep or very large contaminated sites, and ex situ treatment is used for smaller sites or multiple sites needing a central treatment area.

**4.1 Contaminant Sources and Distribution**. The hydrocarbon contaminants may enter the environment as a result of events such as leaks, spills, or discharges at the surface; use in fire training pits; or leaks from underground storage tanks (USTs) or piping. Unless the release occurs innonsorptive, porous soils (e.g., sands), the bulk of the petroleum contaminants entering the environment will not migrate far from the original source. Thus contaminants from leaks, spills, discharges, or fire training pits typically will remain in surface soils where they are accessible for excavation and biopile treatment. Leaks from buried piping or USTs will contaminate deeper soils.

A hydrocarbon release can result in the migration of contaminants to any of four phases in the soil:

- as a sorbed phase on soil surfaces in thevadose zone (soil above the water table)
- in the vapor phase in the vadose zone
- as light, nonaqueous-phase liquid (LNAPL) either floating on the water table or as residual saturation in the vadose zone
- in the water phase dissolved in pore water or groundwater.

A conceptual model of the distribution of hydrocarbon contaminants is illustrated in Figure 6.

Although petroleum contaminants dissolved in the groundwater frequently pose the greatest risk through the drinking water pathway, residual and sorbed LNAPL act as the source for contaminants to the groundwater and vapors. Thebiopile remediation technology reduces risks to human health and the environment by excavation and treatment of soils containing sorbed or residual hydrocarbons.

**4.2 Contaminant Properties**. The success of biopile remediation depends largely on the contaminant characteristics. Petroleum hydrocarbons such as gasoline, diesel fuel, lubricants, or crude oil contain various types of organic compounds as illustrated in Figure 7. Crude oils may contain large fractions of pola**a**sphaltenes and resins containing nitrogen, sulfur, or oxygen. Refined products such as petroleum hydrocarbon fuels and waste oils contain mainly saturated and unsaturated hydrocarbons. Some physical and chemical properties of typical components in petroleum hydrocarbons are compiled in Appendix E.

Gasoline is a fuel consisting of lighter fractions of petroleum. The composition of gasoline typically ranges from 4 to 8% alkanes, 2 to 5% alkenes, 25 to 40% isoalkanes, 3 to 7% cycloalkanes, 1 to 4% cycloalkenes, and 20 to 50% aromatics (0.5 to 2.5% benzene) (U.S. Department of Health & Human Services, 1993a).

Diesel fuel contains mainly hydrocarbons with 10 to 19 carbon atoms. The approximate composition ranges for diesel fuels are 64% aliphatics, 1 to 2% olefins, and 35% aromatics. Gasoline and diesel fuels contain less than 5% polycyclic aromatic hydrocarbons PAHs). Heavier fuel oils contain a higher proportion of hydrocarbons with more carbons in the compound. The heavier fuels also may contain more than 5% PAHs (U.S. Department of Health & Human Services, 1993b).

Jet fuel-4 (JP-4) is a widely used fuel containing petroleum hydrocarbons with carbon chain lengths in the range of 4 to 16. A typical average composition for JP-4 is 32% normalikanes, 31% branchedalkanes, 16%

cycloalkanes, 18% benzenes and alkylbenzenes, and 3% naphthalenes. JP-5 is similar to JP-4 but has a much lower content of benzene, toluene, ethylbenzene, and xylenes (BTEX). Jet fuel-7 (JP-7) is a similar petroleum hydrocarbon fuel but has a narrower boiling range and lower allowed content of aromatic compounds (U.S. Department of Health & Human Services, 1993c).

To be amenable to treatment in abiopile, a compound must biodegrade (a) under aerobic conditions at a rate fast enough to allow for remediation in a few months and (b) at a sufficient rate relative to the vaporization rate to allow for biodegradation to predominate over volatilization. Figure 8 shows the relationship between contaminant vapor pressure and aerobic biodegradability. The data shown in Figure 8 indicate the general trends for biodegradability for a selection of organic contaminants. These data indicate thabiopile treatment is promising for most of the components in jet fuel or diesel fuel based on their pressure and biodegradation rate.

Treatment of soils contaminated with fresh gasoline may require special design provisions due to the high content of BTEX. The BTEX compounds have high biodegradation rates, but they are volatile and more strictly regulated than other components of petroleum hydrocarbons. The actualbiodegradation rates achieved will vary substantially depending on the contaminant concentration, the matrix properties, and the conditions in the biopile. In some cases, treatability testing will be required to evaluate the feasibility obiopile treatment and to determine design parameters. Treatability testing is discussed further in Section 5.1.

**4.3 Soil Properties .** Soils consist of weathered mineral grains and organic materials in varying proportions. Soils typically are heterogeneous and may be stratified due to historical va**xi**tions during the soil formation process. Soil layers form as a result of interactions between the soil and groundwater, atmosphere, and vegetation. The properties of the upper layers are particularly affected by the biological activities of plants and microorganisms. As a result, the surface soil properties are strongly influenced by soil chemistry, moisture content, and climatic conditions.

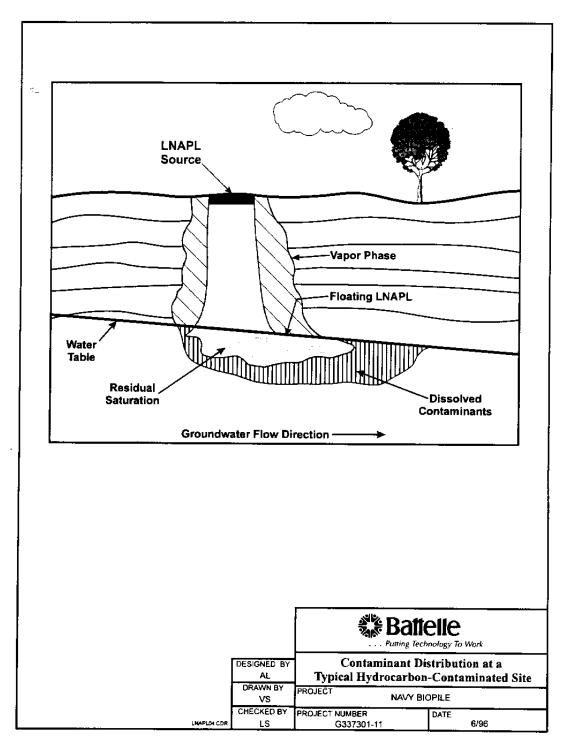


Figure 6. Conceptual Model of the Distribution of Contaminants.

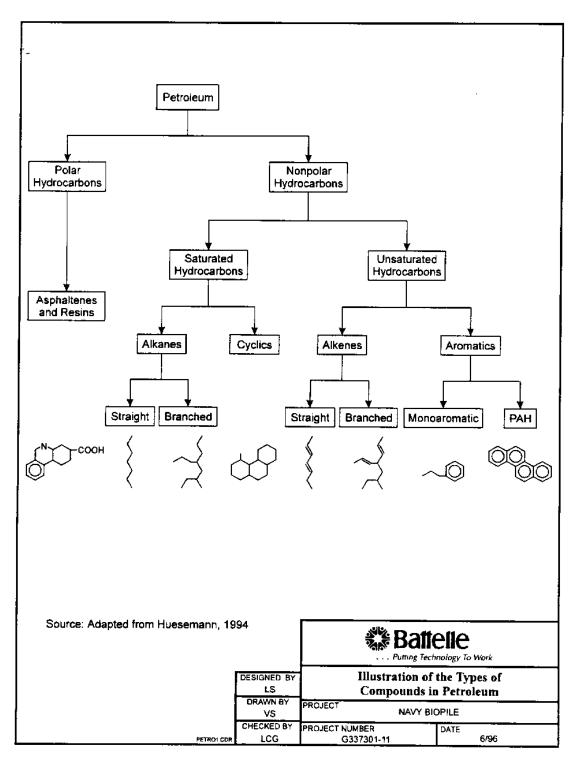


Figure 7. Illustration of the Types of Organic Compounds in Petroleum.

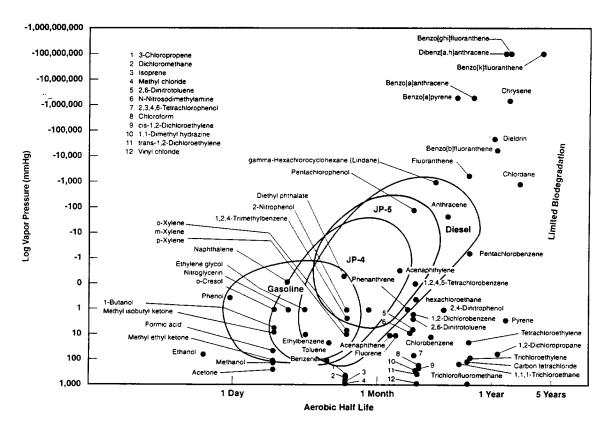


Figure 8. Relationship Between Contaminant Vapor Pressure and Aerobi&iogegradability.

The wide variations in natural soil properties and contaminant distribution encountered at sites cannot be overemphasized. Not only do soil and contaminant conditions vary widely from site to site, but wide ranges of conditions frequently occur within one site. The equipment selected to handle contaminated soils at the biopile site must be able to accept these wide variations.

The permeability and sorptive properties of the soil depend on the soil type. Many systems have been established to classify soil types and constituents. Most of these classifications include particle size as the primary physical parameter. Typical classifications, in order of decreasing size, are: gravel, sand, silt, and clay.

A high clay content in the soil will adversely affectbiodegradation processes in thebiopile in two ways. First, a high clay content will reduce the permeability of the soil and thus increase the difficulty of adequately and uniformly aerating the pile. Also, a high clay content may decrease the availability of the organic contaminants and thus decrease the biodegradation rate because clay strongly adsorbs organic contaminants, reducing the biological availability of the contaminants (Huesemann, 1994).

Movement of air in the biopile occurs primarily through the gas-filled pores. The amount and configuration of soil porosity are important determinants of the gas permeability of the soil. Soils with connected porosity generally are more amenable to treatment in abiopile. The soil permeability or porosity can be increased by adding bulking agents or by soil shredding, if needed.

The organic content of soil can vary from less than 1% in dry, sandy soils to more than 20% in soils that are exposed to water much of the time. The chemistry of the organic portion of soils is complex. The soil organic content will consist of high-molecular-weighthumic materials and lower-molecular-weight organic acids and bases.

The high-molecular-weighthumic materials have low water solubility and high affinity for organic and inorganic contaminants.

**4.4 Influence of Soil Properties on Biopile Treatment .** Soil properties also affect the rate of biodegradation. Biopile remediation is conducted ex situ so amendments can be added prior to forming the pile, and the pile conditions can be controlled to overcome some of the limitations caused by the soil properties. The following soil properties most affect thebiodegradation rate: moisture content, pH, nutrient supply, and metal content.

These soil properties are discussed in Sections 4.4.1 through 4.4.4, respectively.

**4.4.1 Moisture Content .** The soil must contain enough moisture to encourage growth of the hydrocarbondegrading microorganisms, but not so much as to reduce soil permeability. Water is essential for biological processes because it not only provides the transport medium for the chemicals that supply energy and nutrients to the microorganisms but also enables the metabolic processes to proceed. However, excessive moisture will fill the pores in the soil pile and reduce soil permeability, making it difficult to aerate thebiopile. Nevertheless, microorganisms will effectively degrade hydrocarbons over a wide range of moisture contents.

Out of 123 sites surveyed in abioventing field study, the soils at 114 sites contained between 5% to 25% water by weight. A slight increase inbiodegradation with increasing moisture was detected, but the results did not show a strong correlation between thebiodegradation rate and moisture content Leeson and Hinchee, 1995). A biopile would be expected to demonstrate similar behavior with an optimum moisture range of 10% to 20% by weight and 5% to 30% being acceptable.

**4.4.2 Soil pH.** The soil pH may influence thebioremediation process, because soil microrganisms require a specific pH range to survive. Most bacteria function in a pH range between 5 and with the optimum being slightly above 7 (Dragun, 1988). A shift in pH may result in a shift in the microbial population because each species will exhibit optimal growth at a specific pH.

Very few soils will require radical adjustment of the pH prior to forming thebiopile. If soil sampling and analysis indicates that the pH is out of the optimal range, amendments can be intrduced during the initial soil preparation. If the soil pH is too acidic, lime may be added to increase the pH. If the soil pH is too basic, sulfur, ammonium sulfate, or aluminum sulfate may be added to decrease the pH (duesemann, 1994).

**4.4.3 Nutrient Supply .** As in the case of all living organisms, microorganisms must have specific nutrients to sustain a healthy population. The hydrocarbon contaminants present usually provide the carbon and energy sources needed for biological action in thebiopile. Glucose, acetate, or citrate may be added, if an additional supply of carbon or energy is needed to maintain optimal growth (U.S. EPA, 1990, EPA/540/2-90/002). The inorganic nutrients are needed to supplement the basic carbon source and energy source needed to sustain life. Inorganic nutrients required include nitrogen, phosphorus, potassium, calcium, magnesium, sulfur, iron, cobalt, copper, manganese, and zinc.

Most microorganisms can efficiently extract inorganic nutrients for the soil, but their activity may be limited by the availability of some inorganic nutrients. Nitrogen, phosphorus, and potassium are required in the highest concentration and are most likely to be limiting when the carbon source, energy supply, and aeration conditions are satisfactory. Nitrogen, phosphorus, and potassium additions are particularly likely to be needed if the available carbon levels are high.

Although the air supplied to thebiopile and the contaminants is consumed by biological action, the inorganic nutrients are recycled by the ecosystem. As a result, the nutrients do not have to be continually replenished. After the initial inorganic nutrient amendment is made (if needed), no further nutrient additions will be required.

**4.4.4 Metal Content.** High concentrations of metals will retard thebioremediation process. As discussed in Section 4.4.3, trace concentrations of some metals are essential to growth but high concentrations will have a detrimental effect. Other metals, such as arsenic and mercury, have no nutrient value and may reduce biological activity when present at low concentrations. As a general rule, the total transition and heavy metal concentration in soil to be treated should be less than 2,500mg/kg. Higher concentrations of cationic metals may be tolerated if the soil

pH is greater than 6.5 or the cation exchange capacity is high (U.S. EPA, 1983, SW-874).

#### Section 5.0: PREDESIGN ACTIVITIES

This chapter describes activities required to establish the essentiabiopile design features (Figure 9). The predesign activities are carried out to answer such questions as these:

- Does the contaminated soil meet the selection criteria for successful remediation using a biopile?
- If not, can the soil be amended to meet thebiopile technology selection criteria?
- Will the biopile facility be temporary or permanent?
- How much space will the treatment system require?
- What should the project officer consider in selecting a site?
- What are the estimated treatment costs?

The biopile design presented herein has been sized to accommodate 500 yd(382 m<sup>3</sup>) of contaminated soil. This size is sufficient to handle contaminated soil generated from most UST excavations. Theopile can be made larger or additional biopile pads can be constructed to handle larger volumes. Although multiple cells can require somewhat more space than a single, larger cell, this modular approach has several advantages. Compared to designing larger systems, this modular approach enables better soil management with respect to receipt, storage, handling, and amendment of soils; prevents costlyoverdesign while allowing for expansion; and maintains a manageable and securable cover size. Having more than onebiopile pad enables the site manager to process soils in smaller, discrete batches. Furthermore, if one shipment of soil is exceptionally difficult to treat, due to the level of contamination or other reason, it would be isolated from the rest of the soils being processed.

The 500-yd<sup>3</sup> (382-m<sup>3</sup>) size is large enough to process a significant volume of soil, yet small enough to allow two workers to apply and remove thebiopile cover. With larger piles, the plastic cover becomes difficult to install and remove when conducting either moisture addition or soil sampling. Another advantage of **b**iopile limited to approximately 500 yd<sup>4</sup> (382 m<sup>3</sup>) is that a smaller pile is easier to aerate evenly than a larger pile.

5.1 Treatability Studies . This section discusses planning of treatability studies to determine the

#### PREDESIGN ACTIVITIES

·Consolidate data on soil to be treated.

Review treatability study results.

Read and understand provisions of the site permits.

•Develop a permit compliance checklist that highlights required actions and equipment, such as exhaust gas monitoring, site security provisions, etc.

·Calculate nutrient requirements.

Establish whether or not soil shredding and/or blending with a bulking agent will be required.

·Based on total material (soil and, if needed, bulking agent), calculate volume to be processed.

Identify number of biopile pads that will be needed and the approximate size of each.

·Identify labor requirements.

Coordinate support and schedule with base facilities and applicable contractors.

Figure 9. Predesign Activities Checklist.

effectiveness of biopile treatment and to collect information to supportbiopile design. The effectiveness is indicated by the ability of microorganisms in the soil to degrade petroleum hydrocarbon contaminants to acceptable cleanup levels. Design information is gathered through study of the conditions in the biopile environment that enhance the health of fuel-degrading microorganisms. Laboratory information, although not an absolute guarantee of success, can substantially reduce the risk of unexpected costs or poorbiopile performance. All soils received should be analyzed to characterize their contamination and to determine whether they meet the technology selection criteria. In cases where selection criteria are not met and cannot be met using simple adjustments in nutrient or water content, a treatability study may be warranted.

To determine the feasibility of biopile remediation at the intended site, the following initial dataacquisition steps must be carried out:

- Inventory the hydrocarbon type and concentration.
- Measure the population density ofheterotrophic and/or hydrocarbon-degrading microorganisms.
- Measure the pH, nitrogen and phosphorus content, moisture content, and particle-size distribution of the contaminated soil. outlines each of these data requirements for biopile feasibility, indicates the selection criteria, and lists adjustment methods for meeting the selection criteria. An explanation of the sampling and analysis method used for each data requirement is given in Section 6.3.

After analyses of the parameters outlined in Table 1 have been completed, the results should be compared to the biopile selection criteria. If the initial values are consistent with the selection criteria, biopile operation can be considered a valid option for petroleum hydrocarbon contamiant removal. If the initial values are not within the selection criteria, adjustments should be made to bring each design variable to within the selection criteria. Methods for parameter adjustment are indicated in Table 1.

Table 1. Testing Required for Biopile Feasbility

Parameter Measured	Purpose and Comments	Selection Criteria	If Above Criterion Limits	If Below Criterion Limits
TPH and concentration	Determines the nature and concentration of contaminants to be treated	TPH £50,000 mg/kg Low concentration of recalcitrant compounds, such as chlorinated organics, PAHs, and polychlorinated biphenyls (PCBs).	Dilute with uncontaminated soil or add a bulking agent	NA
TPH-degrading microorganism density	Indicates the presence of microorganisms with the potential ability to degrade the contaminants	TPH degraders <sup>3</sup> 1,000 CFU/g dry soil	NA	Verify proper nutrient ratios, H <sub>2</sub> O content, and oxygen delivery; verify that toxic metals are <2,500 mg/kg; consider addition of organisms by way of microbial amendment.
РН	Determines the need for amendments to adjust soil pH	pH @ 6 to 9	Adjust with a cidic compound, e.g., sulfur	Adjust with basic compound, e.g., agricultural-grade lime.
Nitrogen content	Determines the need for amendments to adjust nutrient content	C/N @ 100:15	NA	Add agricultural- grade chemical fertilizer or a nitrogen-containing organic amendment.
Phosphorus content	Determines the need for amendments to adjust nutrient content	C/P @ 100:1	NA	Add agricultural- grade chemical fertilizer.
Moisture content	Determines the need for amendments to adjust moisture content	70% to 95% of field capacity	Allow soil to dry, e.g., use biopile aeration system	Add water to achieve at least 70% field capacity.
Particle-size distribution and particle-size analysis	Indicates the clay content which assists in selecting the type and quantity of bulking agent needed Indicates if soil shredding is appropriate	Low clay or silt content (soil void volume <sup>3</sup> 25%)	NA	Shred soil; add bulking agent.

#### Where deemed necessary, the success of the parameter modifications is evaluated by coducting

NA = Not applicable.

treatability studies in the form of bench-scale flask or column reactor tests. These treatability studies should be conducted in a biological laboratory and should be designed to measure the degradation rate of the petroleum hydrocarbon contamination under aerobic conditions. The effectiveness of any recommended adjustments can be measured by comparing the results of the degradation studies from the "adjusted" soils to the "as received" soils. Several companies specializing in soil-based treatability studies are listed in Appendix F.

Pilot-scale testing provides a relatively large-scale method for evaluating the treatment effetiveness of a given technology applied to given soil conditions. This increase in scale and complexity increases the cost of the treatability study but decreases the uncertainty involved in the seletion and design of the biopile treatment. Under normal circumstances, a pilot-scale test will not be required for evaluating the feasibility of the biopile technology. Pilot-scale tests should be limited to cases where extremely unusual soil characteristics exist or where novel, non-TPH contaminants are involved. If site-specific conditions warrant pilot testing and if the budget allows for the expenditure, a bench-scale and/or pilot-scale treatability study may be conducted. The study could include any or all of the following:

- biodegradation rate as a function of moisture content, nitrogen content, and phosphorus content
- biodegradation rate as a function of soil temperature

specific respiration rate and rate of carbon dioxide evolution (mineralization rate) as a function of adjusted design parameters.

A standard method for abiopile treatability study does not exist. However, well-established procedures are practiced by government and private laboratories, some of which are listed in Appendix F. Determining the optimum parameters for abiopile treatability study may involve laboratory analysis of soil samples or may require setting up a bench-scale or pilot-scale test. Additional information about planning programs for material characterization and treatability studies is provided in two U.S. EPA documents (U.S. EPA, 1992, EPA/540/R-92/071a and U.S. EPA, 1988, EPA/540/G-89/004). In addition, the NFESC document (NFESC, 1996b) cites the purpose for and gives a detailed description of eachbiopile design variable.

A typical sequence for biopile treatability testing would involve preliminary soil screening analyses (as given in the decision tree in Chapter 1), followed by bench-scale testing in flasks or columns, and if necessary pilot-scale testing in a smallbiopile, to reduce the uncertainty to acceptable levels.

Following the initial determination of feasibility, bench-scale testing can be conducted to determine optimum operating conditions. Bench-scale tests can be performed under a range of conditions to study the effects of nutrient amendments, bulking agent addition, temperature changes, or other factors obiodegradation rates. Bench-scale studies can be conducted inbiometer flasks or in soil columns.

A biometer flask provides a controlled environment for a small (80- to 100-gram [2.8to 3.5-ounce]) soil sample. Air, filtered through Ascarite<sup>TM</sup> to remove atmospheric CQ, is slowly purged through the flask to maintain aerobic conditions. The Q<sub>2</sub> and CO<sub>2</sub> concentrations in the headspace are measured periodically to monitor the rate of biological activity.

A soil column test is conducted by placing a soil sample in a glass oplexiglass column that is 2 to 4 inches (5.1 to 10.2 cm) in diameter and 6 to 36 inches (15.1 to 90.6 cm) long. As with the biometer flask, filtered air is slowly purged through the column. The Q,  $CO_2$ , and TPH concentrations in the exit air are measured periodically.

If the soil or contaminants present unusual problems or if the biopile will include new, unproven design features, a pilot-scale test may be appropriate. The pilot-scale tests will explore a narrow range of operating conditions defined by results from the bench-scale testing. The pilot-scale tests should be conducted using a test apparatus that incorporates the main features of the plannedbiopile but uses a smaller volume of soil. The pilot-scale test typically will include extensive sampling and analysis and more on-line monitoring equipment than the production pile.

As stated previously, a pilot-scale test typically is not required to evaluate the feasibility of the biopile technology. A decision to perform more complex or larger scale studies requires a trade-off between the cost to perform the study and the reduced uncertainty provided by the study. Usually this trade-off favors pilot-scale testing if the contamination, the soil, or the site conditions are of an unusual unproven nature.

Performing a treatability study for each unit at a facility may **w**t be appropriate. A decision to perform a treatability study and selection of the scale of the study depend on site-specific conditions. The presence of a limited number of contaminants in a homogeneous matrix tends to reduce the uncertainty about the potential performance of biopile treatment and thus reduces the need for testing. A treatability study may not be needed, if results can be extrapolated from studies on similar soils with similar contaminants and concentrations. Data available from literature sources or studies performed at other locations at the facility may give sufficient confidence to allow selection or design of the biopile treatment option. For example, little or no testing would be required when considering jopile treatment for sandy soils contaminated with low to moderate concentrations of light hydrocarbon fuels, provided that the selection criteria outlined in Figure1 can be met.

The scope of this document is limited to the aerobic treatment of soils contaminated with monchlorinated petroleum hydrocarbons. Other contaminants, such as high-molecular-weighPAHs, pesticides, and chlorinated hydrocarbons, are beyond the scope of this document and would require more extensive treatment studies.

**5.2 Decision Between Temporary and Permanent Site**. Biopiles can be constructed as either temporary or permanent facilities. The main difference between these two options is the design of the soil storage an**d**iopile pads. For temporary biopiles, the pads can be built on an existing asphalt or concrete surface or may be built on a compacted soil or clay foundation. A permanentbiopile would be constructed on a concrete foundation specifically constructed for the biopile. Upgraded support facilities, such as an equipment storage building, a soil storage shelter, improved access roads, etc., are more likely to be constructed for a permanent site.

In most cases, a temporarybiopile facility will be sufficient. However, in situations where the projected facility operating life is 5 or more years, a permanentbiopile facility would be warranted. Other site-specific requirements, such as directives from command authorities or an expected high soil throughput, may dictate the selection of a permanentbiopile. Sites expecting to handle soils from several locations in batches over extended time periods may consider the cost-to-benefit ratio of a permanent versus temporary facility.

The lower cost and the reduced sitecloseout and permitting requirements of a temporarybiopile site usually will compare favorably to the more involved site development and higher capital costs of a permanent facility. Where useful, the project officer may decide to incorporate some of the support features of a permanent site, such as an equipment storage building or an improved surface for material processing, without choosing a concrete pad.

**5.3 Biopile Site Selection**. The biopile site, whether temporary or permanent, must be accessible, have access to utilities, be flat on solid ground, be located beyond the 100-year flood plain, be securable, and not be within a residential area. Selecting a site that has existing facilities can reduce costs and site preparation time. Useful amenities could include a usable asphalt or concrete pad (such as a parking lot), covered area for soil and equipment storage, utilities, and a secured perimeter. Table 2 lists suggested guidelines for selecting **b**iopile site.

Early in the site selection process, the project manager should ask base authorities for suggestions on where to locate the biopile facility. Regulatory officials also may have constructive input in the site selection process. Receiving timely guidance could reduce the risk in choosing a location. After deciding on a potential location, a site walkthrough should be organized to include staff from the base facilities and environmental departments. During the walkthrough, available utilities and other amenities can be identified, and shortcomings and possible upgrades can be noted. The walkthrough also can assist the project officer to develop a tentative site layout that can be presented in the work plan to base and regulatory officials. Once final approval has been granted, the work plan can be finalized and site preparation can begin.

When choosing a site, consider the following points as part of the selection process:

- Choose a centralized site for soil handling.
- Avoid off-base soil hauling, because it is hard to get public approval for a truck route.

# • Choosing a site with a slight slope is acceptable, because this may be desirable for Table 2. Biopile Site Selection Guidelines

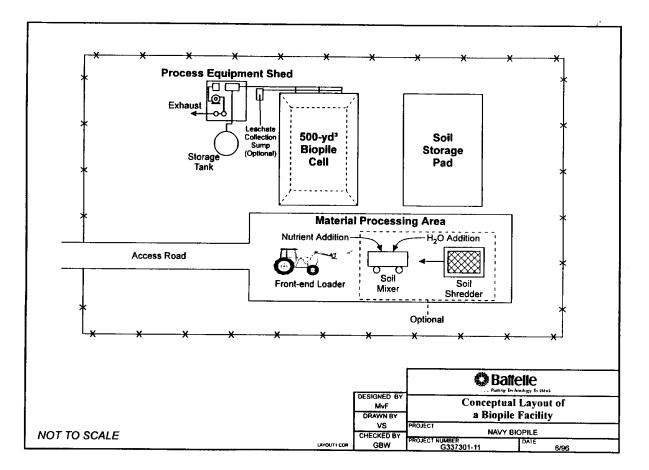
Selection Parameter	Definition	Recommendation
Geography	Location and natural and improved site conditions	Select a flat area with good drainage located outside the 100-yearfloodplain. Site should be located a reasonable distance (~500+ yd) from residential areas. Existing improved site, such as a parking lot or vacant storage yard, would be attractive.
Accessibility	Approach to and exit from site	Roadway(s) to site should be improved gravel, asphalt, or concrete. Hard dirt may be acceptable in arid climates. Ensure load limits at bridges meet hauler's requirements. Locate site close to contaminated soil source, if possible. Ensure site will be available for duration of project.
Space requirements	Area required to operate biopile facility	Includes access/egress road, soil storage area, processing area,biopile, pump shed, storage tank, and buffer zone.
Utilities	Electricity and water sources	For single biopile pad, 110/220-V, 60-amp lines should be sufficient. Check with electrical shop for actual requirements. Water source should be present on site.
Soil logistics	Transport, handling, storage of soil	Delivery trucks need adequate access route and space to maneuver on site. Soil handling equipment should be available for any required moving, mixing, and shredding of soil. Contaminated soil area must have an impermeable, bermed liner pad. Stored soil should be protected from the weather using a waterproof cover, or by placing soil under a roof.
Security	Site access control measures	Fenced-in area with gated access. Place sign at gate citing project type and point of contact. Lock storage and pump house.

biopile drainage.

• Choosing a site with nearby sources of power and water is very desirable.

**5.4 Sizing the Site.** The quantity of contaminated soil to be received, frequency of reception, expected retention time in the biopile, and soil preparation requirements will dictate the area required for storage and treatment. The site geography and existing facilities, such as buildings, pavement, fencing, etc., must be taken into account along with the net storage and treatment areas required to calculate the area and map the overall site boundaries.

For a 500-yd<sup>a</sup> (382-m<sup>3</sup>) biopile system, the net treatment area will consist of the 50 ft× 60 ft (15.2 m × 18.3 m) biopile pad; the associated footprints of the pump shed,leachate collection tank, and off-gas treatment unit; the soil storage area; space to maneuver soil-handling equipment; and space to prepare the soil (if needed). The 50 ft 60 ft (15.2 m × 18.3 m) biopile cell can process a soil volume ranging from 250 to 750 yd(191 to 573 m<sup>3</sup>), with 500 yd<sup>a</sup> (382 m<sup>3</sup>) being the median design volume. The typical pump shed is 8 ft× 10 ft. The soil storage pad could feasibly be smaller than the biopile pad, because the soil can be stacked higher on the storage pad. An estimated soil storage area



with a 500-yd<sup>a</sup> (382-m<sup>3</sup>) capacity would be 1,700 f<sup>e</sup> (158 m<sup>2</sup>), assuming a 10-ft (3-m) height. Figure 10 depicts abiopile system with a 500-yd<sup>a</sup> median capacity.

Figure 10. Plan View of a Biopile Site Layout.

Taking into account the net treatment and storage space requirements, an estimated size for a 500-y<sup>d</sup> (382-m<sup>3</sup>) biopile facility would be 11,000 ff (1,022 m<sup>2</sup>), plus an access road. As shown in Figure 11, facilities having more than one 500-y<sup>d</sup> (382-m<sup>3</sup>) module could use a shared soil processing area. Assuming a common soil staging and processing area and overlapping space for equipment maneuvering, a 3,000-y<sup>d</sup> (2,294-m<sup>3</sup>) biopile facility would require a minimum of 40,000ft<sup>2</sup> (3,716 m<sup>2</sup>) for the entire site. At some sites, the existing fixed facilities or geography may add to the total site size. For permanentbiopile facilities, additional space would be needed to erect additional buildings, for example, to serve as a control center or to store equipment.

The BPCE is based on the designs presented in this manual and takes into account installation and operating requirements. Operating requirements include weekly and monthly services, sampling and analysis, rebuilding of the biopile when multiple cycles are required, and sitecloseout costs. Design assumptions made in the BPCE to estimate the treatment cost per cubic yard are as follows:

- Land is available at no cost.
- A sufficient soil storage area already exists.
- Permitting has been completed.
- Soil is transported to and from site at no cost.
- Utilities are provided at no cost.
- No contingency is made for replacement parts.
- No soil shredding or bulking agent addition is required.
- Labor rates include all applied overhead costs.
- The treatment cell dimensions are 50ft  $\times$  60 ft (15.2 m  $\times$  18.3 m) with a mean treatment capacity of 500 yd<sup>3</sup> (382 m<sup>3</sup>) and a maximum soil capacity of 750 yd<sup>3</sup> (573 m<sup>3</sup>).
- The aeration system operates in the extraction mode with off-gas treatment.
- No irrigation system is required.
- 2,900 lb (1,315 kg) of urea and 400 lb (181 kg) of diammonium phosphate are added per cell as nutrients.
- The processing time per cycle is 4months.

In general, the cost to treat a unit volume of soil will decrease as the number of treatment cycles per cell increases. The cost of a permanenthiopile facility generally begins to become comparable with respect to the cost of a temporary facility when the total soil volume to be treated exceeds 5,000/d<sup>3</sup> (3,823 m<sup>3</sup>). In general, a permanent facility should be considered if the expected project life is 5years or more. However, site-specific conditions may dictate whether to use a temporary or permanentbiopile design, irrespective of the expected total volume or project life.

Taking into account the net treatment and storage space requirements, an estimated size for a 500-yd<sup>3</sup> (382-m<sup>3</sup>) biopile facility would be 11,000 ft<sup>2</sup> (1,022 m<sup>2</sup>), plus an access road. As shown in Figure 11, facilities having more than one 500-yd<sup>3</sup> (382-m<sup>3</sup>) module could use a shared soil processing area. Assuming a common soil staging and processing area and overlapping space for equipment maneuvering, a 3,000-yd<sup>3</sup> (2,294-m<sup>3</sup>) biopile facility would require a minimum of 40,000 ft<sup>2</sup> (3,716 m<sup>2</sup>) for the entire site. At some sites, the existing fixed facilities or geography may add to the total site size. For permanent biopile facilities, additional space would be needed to erect additional buildings, for example, to serve as a control center or to store equipment.

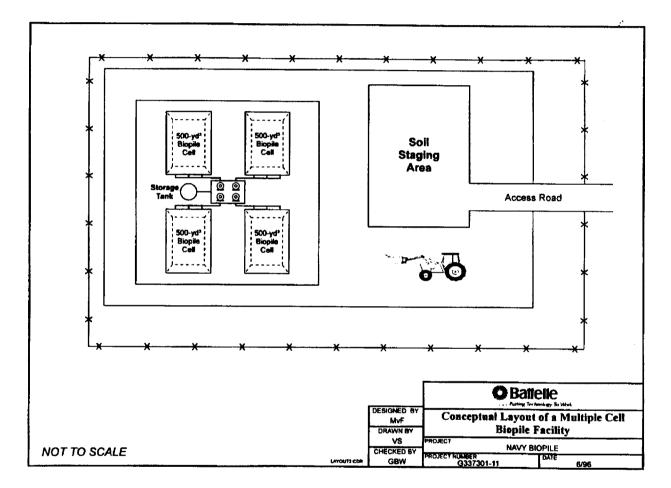


Figure 11. Plan View of a Site with Multiple Biopile Modules.

**5.5** Cost Analysis. To provide the reader with an idea of the cost of a biopile system, various cost models were run using the Biopile Cost Estimator<sup>®</sup> (BPCE) software, Version 1.21, developed by Battelle for the Naval Facilities Engineering Service Center (NFESC, 1996a). The parameters and resultant costs for each case calculated are summarized in Table 3. Appendix H contains cost summary sheets for each case. Appendix H also gives examples of the detailed cost sheets for the installation and

operation and maintenance cost estimates for the temporary and permanent baseline biopile cases treating a total of  $500-yd^3$  (382-m<sup>3</sup>) of soil.

Biopile Type	Total Volume (yd <sup>3</sup> )	No. of Treatment Cells	Volume per Cell (yd <sup>3</sup> )	No. of Cycles	Project Life (months)	Cost (\$/yd <sup>3</sup> )
Temporary	500	1	500	1	4	103.49
	1,000	1	500	2	8	75.58
	2,000	1	500	4	16	63.26
	2,000	2	500	2	8	69.70
	5,000	1	500	10	40	56.65
	5,000	1	750	7	28	40.71
	5,000	2	500	5	20	56.54
	5,000	2	750	4	16	46.26
Permanent	500	1	500	1	4	196.99
	5,000	1	500	10	40	66.30
	5,000	1	750	7	28	50.36
	10,000	1	500	20	80	61.47
	10,000	2	500	10	40	62.71
	20,000	2	500	20	80	58.27
	20,000	4	500	10	40	62.45
	40,000	4	750	14	56	41.34

Table 3. Summary of the BPCE Cost Analyses for a Biopile Construction and Operation

#### Section 6.0: BIOPILE CONSTRUCTION

In Section 2.0, a general biopile technical overview is presented. This section is focused on specific biopile design. Section 6.1 describes the construction of a temporarybiopile. Section 6.2 describes additional design considerations for building a permanentbiopile.

The baseline biopile design presented herein has been sized to accommodate 500 ytl(382 m<sup>3</sup>) of contaminated soil. This size is sufficient to handle contaminated soil generated from most UST excavisions. Additional biopile pads can be constructed to handle larger volumes. Although multiple cells can require somewhat more space than a single, large cell, the modular approach has several advantages. This approach enables better soil management with respect to receipt, storage, handling, and amendment of soils. Having more than on**b**iopile pad enables the site manager to process soils in smaller, discrete batches. Furthermore, if one shipment of soil is exceptionally difficult to treat, due to the level of contamination or other reason, it would be isolated from the rest of the soils being processed.

The 500-yd<sup>3</sup> (382-m<sup>3</sup>) size is large enough to process a significant volume of soil, yet small enough to allow two workers to apply and remove thebiopile cover. With larger piles, the plastic cover becomes difficult to install and remove when conducting either moisture addition or soil sampling. Another advantage of **b**iopile limited to approximately 500 yd<sup>4</sup> (382 m<sup>3</sup>) is that a smaller pile is easier to aerate evenly than a larger pile.

The phases involved in constructing abiopile include:

- site preparation
- base construction
- aeration system installation
- nutrient addition
- moisture addition system
- leachate collection system installation (optional)
- pile formation.

Each of these phases is described in Section 6.1. Design considerations for permanenbiopile sites are covered in Section 6.2.

**6.1 Temporary Biopile Construction.** Once the biopile site has been selected and the proper permits have been obtained, the biopile construction can begin. The number ofbiopile pads and the size of the soil storage area need to be calculated based on the volume of soil to be treated at the site per 3- to 6-month period. Figure 12 is a general checklist that summarizes the majorbiopile construction activities. Figure 13 is an example list of parts needed for building one 500-yd<sup>3</sup> (382-m<sup>3</sup>) temporary biopile module.

BIOPILE CONSTRUCTION CHECKLIST	
Order materials	
Construct:	
soil storage pad foundation	
soil storage pad	
biopile pad foundation	
biopile pad	
pump pad/shelter	
water knockout system	
off-gas treatment system	
aeration system	
irrigation system (optional)	
leachate collection system (optional)	
Ensure power and control circuits are installed properly	
Form biopile	
Install cover	
Receive and cover contaminated soil on storage pad or place directly orbiopile pad if it is completed	
Construction requirements:	
minimum two laborers and one front-end loader operator	
minimum heavy equipment: one front-end loader	
additional soil blending equipment (optional) water source	—
	—
nutrients health and safety equipment as distand in the Health and Safety Plan	
health and safety equipment as dictated in the Health and Safety Plan	
Conduct time-zero sampling	—
See parts list in Figure 13 for related materials and equipment.	

Figure 12. Biopile Construction Checklist

# EXAMPLE PARTS LIST

treated 4-inch  $\times$  4-inch lumber

treated 2-inch  $\times$  4-inch lumber 3/4-inch  $\times$  6-inch lag bolt w/washer and nut to secure 4-inch  $\times$  4-inch beams connecting brackets to join 4-inch  $\times$  4-inch beams 60-mil (or thicker) 51-ft  $\times$  61-ft HDPE bottom liner

3/8-inch  $\times$  3-inch hex head sheet screw w/washer @ 100 ea.

clean dirt or clay for 8-inch foundation @ 71 yd 3 4-inch flexible, slotted drainage pipe @ 90 ft 4-inch end caps for drainage pipe @ 3 ea. 4-inch schedule 40 PVC pipe @ 30 ft 4-inch rubber unions to connect drainage pipe to PVC @ 3 ea. 4-inch brass gate valves @ 3 ea. 4-inch schedule 40 PVC threaded/slip coupling @ 6 ea. 4-inch to 2-inch PVC reducing bushing @ 3 ea. 2-inch PVC slip tee @ 2 ea. 2-inch PVC slip elbow @ 2 ea. (will vary depending on site layout) 2-inch schedule 40 PVC pipe @ 60 to 80 ft (adjust as dictated by proximity of pump) 5-gal water knockout tank with automatic level control 1-inch schedule 40 PVC pipe @ 20 to 40 ft (adjust as required to plumb from knockout tank to water collection tank) miscellaneous 1-inch PVC fittings as required 500-gal water collection tank 1<sup>1</sup>/<sub>2</sub>-hp rotary positive displacement vacuum pump @ 1 ea. concrete pad or other solid base for pump foundation storage shed to house pump and materials 55-gal granular activated carbon drum @ 2 ea. miscellaneous PVC fittings to connect knockout tank, pump and carbon drums plastic sheeting (HDPE or qualified substitute) to serve as top cover @ 75  $\times$  75 ft 2-inch  $\times$  4-inch  $\times$  6-ft treated wood slat for securing cover @ 30 ea. 5/16-inch  $\times$  4-inch hex head sheet metal screw @ 100 ea. nylon rope @ 400 ft 5/16-inch  $\times$  3-inch eye-screw @ 12 ea. nutrients (e.g., urea and diammonium phosphate or calcium phosphate) to achieve 100:15:1 carbon-to-nitrogen-to-phosphorus ratio water hoses with nozzles scoop for nutrient addition scale to measure nutrients (optional) 1-gal bucket for holding/distributing nutrients (optional) <sup>1</sup>/<sub>4</sub>-inch nylon tubing for monitoring points green @ 100 ft; red @ 300 ft; blue @ 300 ft monitoring point screen @ 10 ea. 1/4-inch brass quick-disconnect coupling set for monitoring points @ 13 ea. thermocouple wire w/plug, 40-ft length @ 2 ea. K-type thermocouple wire w/plug, 20-ft length @ 2 ea. shovel @ 2 ea. health and safety equipment as specified in the health and safety plan duct tape @ 1 roll

Figure 13. Example Parts List.

**6.1.1 Site Preparation.** The first step in site preparation is to evaluate the site with respect to material flow, access to utilities, site entrance and exit, existing security measures, and general layout of space (Figure 14). For the standard 500-yd<sup>a</sup> (382-m<sup>3</sup>) module, the dimensions of thebiopile pad will be 50 ft  $\times$  60 ft (15.2 m  $\times$  18.3 m) with a pile height ranging from 5 to 6 ft (1.5m to 1.8 m). The storage pad dimensions can vary according to site limitations and the actual soil volume to be treated.

SITE PREPARATION ACTIVITIES
Develop a site preparation and construction schedule.
Prepare Health and Safety Plan and obtain required approvals.
Coordinate with base environmental and facilities staff to establish construction start date and vase support
a d to determine what facilities are available.
Conduct a walk through survey of site with base facilities representative to visua lly identify site assets, space,
and deficiencies.
Contact electrician to install/modify electrical power as necessary.
Have base facilities support install waterline with spigot, if one is not available.
Clear debris in and around site.
Install security measures, such as a fence with gated access and appropriate signs.
Grade area for soil storage pad and biopile pad(s).
Set up soil processing equipment if soil will need screening, blending, or shredding.

Figure 14. Site Preparation Checklist.

For example, sites with total soil volumes of less than 500 yd (382 m<sup>3</sup>) would require a slightly smaller pad. Sites with a total volume between 500 and 750 yd (382 m<sup>3</sup> and 573 m<sup>3</sup>) could use the same base pad dimensions but would be higher (up to 8 ft [2.4 m]).

When laying out the site, the movement requirements of trucks hauling soil, the front-end loader, and any mixing equipment need to be considered. If the treatability study indicated the need for soil shredding, bulking agent addition, or soil mixing, an area to accommodate the required equipment and material processing also should be identified.

Before construction of the storage andbiopile pads can begin, the site is cleared of brush, debris, and other obstacles. If the site contains an existing improved surface, such as an asphalt or concrete area, this surface can be used as the foundation for the staging area, and for the storage andbiopile pads. If there is no improved surface, the foundations for the soil storage area and thebiopile need to be constructed by grading and compacting clean soil or clay.

As soon as the site has been identified, the site manager should arrange for installation of power and water utilities if they are not already in place. The electrical power input must be suffiient to service the required blower(s). For a single 500-yd (382-m<sup>3</sup>) biopile, a 110/220-V, 60-amp, single-phase circuit to power a 1½-hp (1,120W) blower will be sufficient. Additional blowers would require an increase in the voltage or amperage requirements. The local electrical support shop or contractor will be able to recommend the exact power needs for the site. Addiionally, a 110-V single-phase line should be in place to power the sump pump from the collection tank and the aeration line water knockout tank pump. Miscellaneous control units, recorders, and remote sampling and data acquisition units may need a 110-V power source. Water will be required at the site to hydrate the soil.

**6.1.2 Biopile Base Construction.** The biopile base consists of a soil or clay foundation, impermeable liner, leachate containment berm, and clean soil base. The optimal thickness of the soil or clay foundation ranges from 6 to 10 inches (15.2 to 25.4 cm) of loose soil that should be compacted to between 80 and 85%. Where possible, an existing asphalt or concrete surface, such as a parking lot, can serve as the biopile and storage area foundations, in lieu of the

compacted soil or clay. Ideally, the graded foundations for the storage area and theoipile should be smooth with approximately a 1- to 2-degree slope toward theleachate collection drain line or sump located at one corner of the biopile. The foundation should extend approximately 3 ft (0.9m) beyond the biopile base to allow for the emplacement of the aeration manifold header, any irrigation lines, and theoipile berm.

Once the foundation has been  $\pounds$  lected and formed, the impermeable liner is placed over the base. The liner typically is a thick plastic material such as 40- to 60mil high-density polyethylene (HDPE). The liner must be large enough to cover the desired 50× 60 ft (15.2×18.3 m) base and to enable secure attachment to the side or top of the biopile berm. The liner can be fastened to theberm using lag screws with washers. Figure 15 outlines the construction design of a typical temporarybiopile; Figure 16 shows a plan view of thebiopile construction.

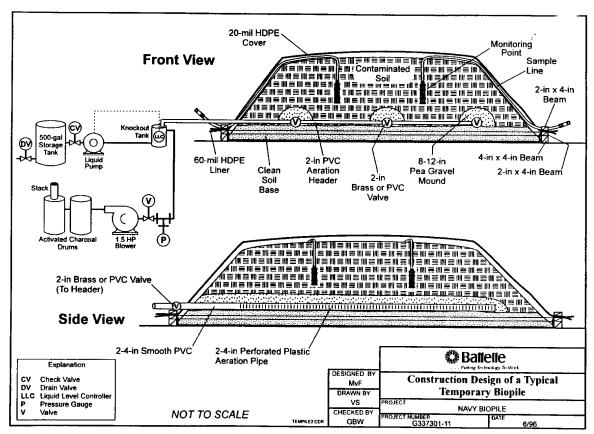


Figure 15. Construction Design of a Typical TemporaryBiopile.

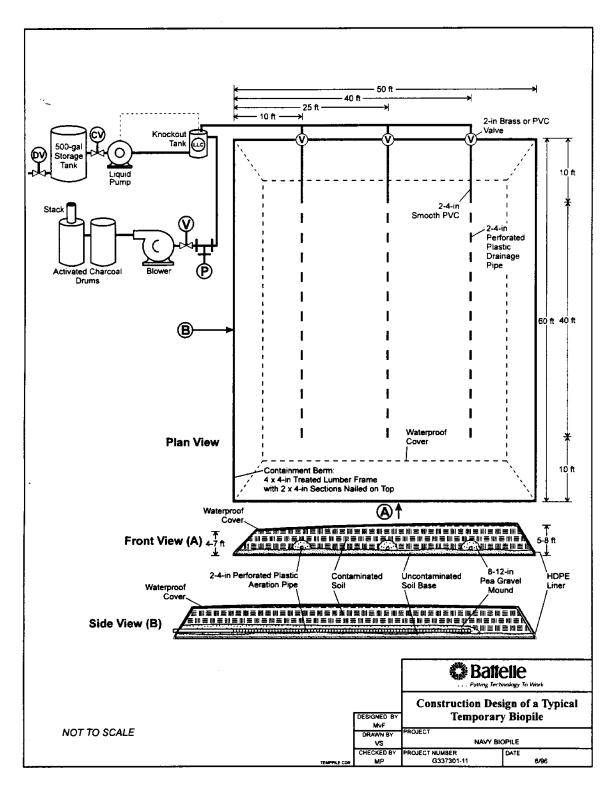


Figure 16. Construction Design Detail of a Temporary Biopile.

The leachate containment berm framing the biopile liner can be constructed using one layer of treated wood 4-inch  $\times$  4-inch beams topped with a layer of treated wood 2inch  $\times$  4-inch or a second layer of 4-inch  $\times$  4-inch beams. The 4-inch  $\times$  4-inch beam sections can be joined using metal brackets and hex head lag screws. The second layer of wood can be connected to the bottom layer with either brackets or by drilling screws directly into the bottom layer. When constructing theberm, it may be useful not to attach the side opposite the aeration manifold header to the other two berm sides, so that it can be removed during the construction of thebiopile. Having the side opposite the manifold header relatively flush with the foundation will enable easy access to thebiopile base with the front end loader as it transports clean soil for the soil base construction and contaminated soil from the storage area for the pile construction. Temporarily removing the oneberm side will eliminate the necessity and effort of building and removing a soil ramp for the loader to drive onto thebiopile base pad. Once all the soil has been loaded on thebiopile pad, the removed berm wall can be replaced and secured.

After the liner and berm have been emplaced, the 8- to 10-in. (20.3- to 25.4-cm) clean soil base can be placed over the liner. The clean soil base serves to protect the liner during the biopile construction and removal. The clean soil base should be compacted to prevent short-circuiting of the air flow.

**6.1.3 Aeration System.** Air can be pushed or pulled through thebiopile with a blower. Injecting (pushing) air is preferred, because the blower does not need to be preceded by a water knockout system to protect it from exhaust gas condensate and possiblebiopile leachate. However, in cases where exhaust gas treatment or deachate collection system is required, the aeration system will need to operate in the extraction mode.

The basic aeration system components are an aeration pump, an air manifold with a header pipe connected to the pump, and valves at the manifold branch points. When operating in the extraction mode, a water knockout tank, cyclone separator (optional), knockout water collection tank, and exhaust gas treatment unit (optional) are added to the aeration system.

The aeration pipes are placed on the clean soil base and lead back to the manifold header. Each aeration leg is joined to the manifold header via a gate valve. The valve is used to adjust the airflow through each leg. The header pipe leads to a water knockout tank to separate the bulk of the water carried through the header from the biopile. Water from the knockout tank can be pumped to a 500-gal (1,895-L) water collection tank, which can be the same as the leachate collection tank, if one is used (see Section 6.1.6). Following the water knockout tank, a cyclone separator can be installed, if deemed necessary, to remove remaining moisture droplets still entrained in theippile exhaust. Removing water droplets from the exhaust gas is important when a carbon absorption unit is used to treat volatile organic carbon (VOC) emissions and helps to protect the blower. Although a relative humidityr[H) of up to 90% will not be detrimental (75% max.rH is ideal), free water droplets can cause the activated carbon to lose effectiveness as it becomes saturated with water. The activated carbon canister size can be obtained from a vendor of activated carbon.

If during the initial feasibility analysis and permitting process exhaust gas treatment was deemed necessary, two activated carbon canisters can be installed in series to remove/OCs. Duplicate carbon canisters are installed to ensure continued off-gas treatment should the first canister reach the contaminant breakthrough stage. To monitor the continued efficacy of the off-gas treatment unit, a sampling port can be installed between the two canisters for periodic off-gas monitoring. If off-gas treatment is not necessary, the aeration system can be run in the extraction mode without carbon canisters, or it can be run in the injection mode. If run in the injection mode, the knockout tank, water collection tank, and off-gas treatment system are not needed. When running in the injection mode, extra caution must be taken to ensure the cover is properly secured.

Each 2- to 4-inch (5.1- to 10.2-cm)-diameter aeration leg should be constructed from one 10-ft (3-m)-long section of blank PVC pipe and one 30-ft (9-m)-long section of slotted, corrugated, and flexible drainage pipe or slotted PVC. The drainage pipe is capped at the end. A rubber union can join the flexible pipe to the PVC pipe. The PVC pipe is connected to the 2-inch (5.1-cm) valve that leads to the 2-inch (5.4cm) manifold header pipe. The distance between aeration pipes should be 8 to 10 ft (2.4to 3.0 m). To prevent short-circuiting of the airflow through the pile, the slotted portions of the aeration legs should be placed toward the center of the pile (i.e., the comection between the blank and the slotted pipe should be approximately 10 ft (3m) from the biopile berm). Figure 17 shows a schematic of the aeration system configured to operate in the extraction mode.

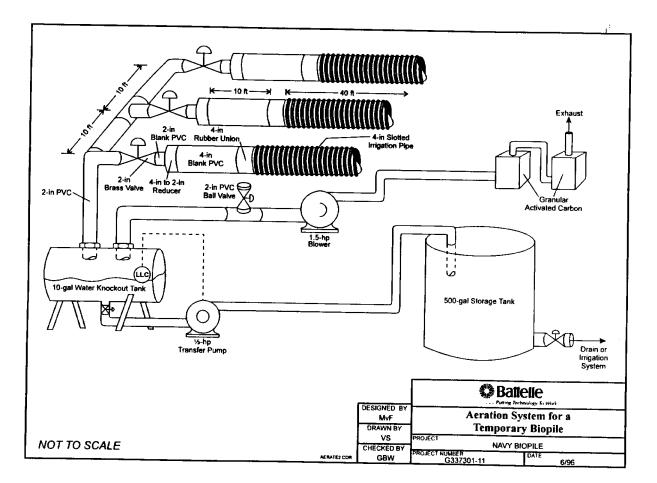


Figure 17. Aeration System for a TemporaryBiopile Configured to Operate in the Extraction Mode.

The aeration source that either pushes or pulls air through the pile should be a 1½-hp regenerative centrifugal (preferred) or rotary positive-displacement blower capable of drawing approximately 120 scfm (3,400 sLm). The blower should be placed on an improved, level surface, preferably a concrete pad. Power requirements for the blower, generally single-phase 110/220-V, will be specified by the vendor and in the blower installation manual. A tee fitting with a ball valve attached to the unconnected end should be installed just prior to the blower inlet. The valve can be used to regulate airflow through the pile (Figure 17). Housing the pump in a shed will protect it from the weather. The same shed can serve as a storage area for materials.

**6.1.4 Nutrient Addition.** Nutrients (nitrogen, phosphorus, potassium) are sold as commercial fertilizer in bag or bulk form. Nutrients can be added by dissolving them in water and spraying the nutrient solution into the soil during the mixing process, or they can be added as dry powder or granules. If no soil mixing is conducted, nutrients can be added as the soil pile is formed. When adding nutrients to the pile as a solution in water, care must be taken that the nutrient solution soaks into the soil and does not leave the soil as runoff. For soils that are not absorbent or that form a crust when wetted, dry nutrient addition during pile formation may be more effective.

Nutrient addition should be controlled by calculating the nutrient requirements and applying the nutrients at a known rate to the soil being piled on the biopile pad. Figure 18 shows a nutrient addition calculation worksheet that can be used to determine the amount of nutrients to add per quantity of soil (in this illustration pounds of nutrient per cubic yard of soil). Appendix J gives a detailed sample calculation for nutrient addition requirements.

When applying nutrients as a solution, the application rate can be regulated by knowing the nutrient concentration levels in the solution, the solutionflowrate, and the volumetric application rate of the soil by the loader. The nutrient solution can be formed in theleachate collection tank and applied using a sump pump and garden hose. The flowrate can be calculated by timing how long it takes to fill a 5-gal (19L) bucket with the nutrient solution. The soil application rate is merely the number of buckets the loader applies to the pile per unit time.

When applying dry nutrients to the soil, they can be weighed in a bucket and applied to the proper amount of soil. An easy way to apply dry nutrients is to measure the amount required per bucket and add that amount with each scoopful of soil.

**6.1.5 Moisture Addi tion**. Having a sufficient moisture content in the pile, generally 70 to 95% of field capacity, is one of the critical factors for successful contaminantbiodegradation. In moderate climates, abiopile generally loses 1 to 2 weight % of the original water. Therefore, if the biopile is sufficiently hydrated when the pile is constructed, no irrigation system usually will be required. The water can be added in three ways:

- Hydrate the soil while it is still on the storage pad.
- Add water as the pile is being constructed.
- Install a dripline irrigation system across the top of the pile.

	<b>BIOPILE NUTRIENT ADDITION WORKSHEET</b>
1.	Nutrient Source:         a.       Nitrogen source (e.g. urea) weight fraction nitrogen (urea = 0.46) <sup>a</sup> b.       Phosphorus source (e.g. diammonium phosphate) weight fraction phosphorus         c.       Potassium source (e.g. potassium sulfate) weight fraction potassium
2.	Total organic carbon content insoil: mg/kg dry soil. Obtained from laboratory results. If unknown, calculate as below:         a. Average concentration of hydrocarbon contamination in soil = mg/kg dry soil         b. Average carbon content in contamination = line 2a×0.8 = mg carbon/kg dry soil
3.	Desired C:N:P:K ratio. Determine by treatability tests, else use C:N:P:K = 100:15:1:1.
4.	Amount of nutrient to add per kg of dry soil. (If not known, assume negligible N,P,K content in soil prior toutrient addition.)
	a. Nitrogen (N) needed to be added per kg dry soil = line $2b \times 0.15 = \m \text{ mg N/kg soil}$
	<ul> <li>b. Phosphorus (P) needed to be added per kg dry soil = line 2b×0.01 = mg P/kg soil</li> <li>c. Potassium (K) needed to be added per kg dry soil = line 2b×0.01 = mg K/kg soil</li> </ul>
5.	Bulk density of soil = kg/m <sup>3</sup> . <sup>b</sup> (Assume 1,400 kg/m <sup>3</sup> if unknown.)
6.	Nutrients required per m <sup>3</sup> of soil: a. kg N/m <sup>3</sup> soil = line 4a.×line $5 \div 1,000,000 =k \text{g N/m3 soil}$ b. kg P/m <sup>3</sup> soil = line 4b.×line $5 \div 1,000,000 =k \text{g P/m3 soil}$ c. kg K/m <sup>3</sup> soil = line 4c.×line $5 \div 1,000,000 =k \text{g K/m3 soil}$
7.	Pounds of nutrients required per cubic yards of soila.lb N/yd³ soil = line $6a. \times 1.69 = $ lb N/yd³ soilb.lb P/yd³ soil = line $6b. \times 1.69 = $ lb P/yd³ soilc.lb K/yd³ soil = line $6c. \times 1.69 = $ lb K/yd³ soil
8.	Total volume of soil to be treated bybiopile: yd <sup>3</sup>
9.	Pounds of nutrient source to be added per cubic yard of soil: line 7a. ÷ line 1a. = lb of N source required/yd soil line 7b. ÷ line 1b. = lb of P source required/yd soil line 7c. ÷ line 1c. = lb of K source required/yd soil
10.	Total pounds of nutrient sources required for thebiopile: line $9a \times line 8 =$ lb of N source <sup>(c)</sup> to be purchased line $9b \times line 8 =$ lb of P source to be purchased line $9c \times line 8 =$ lb of K source to be purchased
(a)	Weight fraction = $\% \div 100$ .

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Figure 18. Biopile Nutrient Addition Worksheet

<sup>(</sup>a) Weight flaction = 76 + 100.
(b) 1 kg/m<sup>3</sup> = 1.688 lb/yd<sup>2</sup>.
(c) Assumes all N comes from a single source. NA = not applicable.

While the soil is still on the storage pad, it can be hydrated using a hose and a sprinkler or by digging holes partially into the pile with a hand auger and filling the holes up with water. Irgating the pile from the top with a sprinkler may cause excessive runoff if the soil crusts over or is too tight to rapidly absorb the water. Water added in hand-augured holes will drain through the soil to achieve more thorough hydration than achieved by top irrigation.

Adding water as the pile is being constructed may or may not be an effective method. If the soil is being shredded or blended, a precise amount of water can be added per batch of soil prcessed. However, if the soil is being piled rapidly onto the pad, the amount of water that can be added between loader buckets will provide only a fraction of the total water needed. Nevertheless, spraying water onto the soil as it leaves the bucket does add an appreciable amount of water evenly to the soil at all pile depths.

The amount of water to add can be calculated by knowing the moisture content of the soil before construction, the flowrate of water from the water hose, and the volume of soil contained in one loader bucket. The amount of time water needs to be added per bucket is equal to the water needed per bucket of soil divided by the total water flowrate. The waterflowrate from the hose can be calculated by timing how long it takes to fill a 5-gal (19-L) bucket with water. Dividing that time by 5 gives the waterflowrate in gallons per minute (gpm).

Initial soil samples will have been collected and analyzed by a laboratory for several variables, including moisture content. However, the actual variable needed to construct theorem of water-holding field capacity to which the soil is hydrated. The desired value is 70 to 95% of field capacity. The field capacity can be determined in the laboratory using ASTM D2365 or ASTM 3152. Methods for estimating the field capacity of a soil are shown in Appendix G.

At most sites, it may not be necessary to hydrate the pile following initial water addition. After completion of construction, if needed, water can be added via adripline irrigation system (Figure 19). Thedripline system applies water at a low rate to prevent the formation of water pools and the subsequent runoff that would result from using higherflowrates. The dripline can be designed so that water is dispersed uniformly across the biopile surface. The waterflowrate through the dripline system can be measured with a simplerotameter. Once the desired amount of water has been added to the biopile, the dripline can be turned off. The dripline system has three advantages:

- It is a convenient and effective method to add water to thebiopile after the pile has been covered.
- The low water flowrate minimizes water runoff from pile.
- The system can operate without supervision.
- Irrigation by this method can achieve even water distribution.

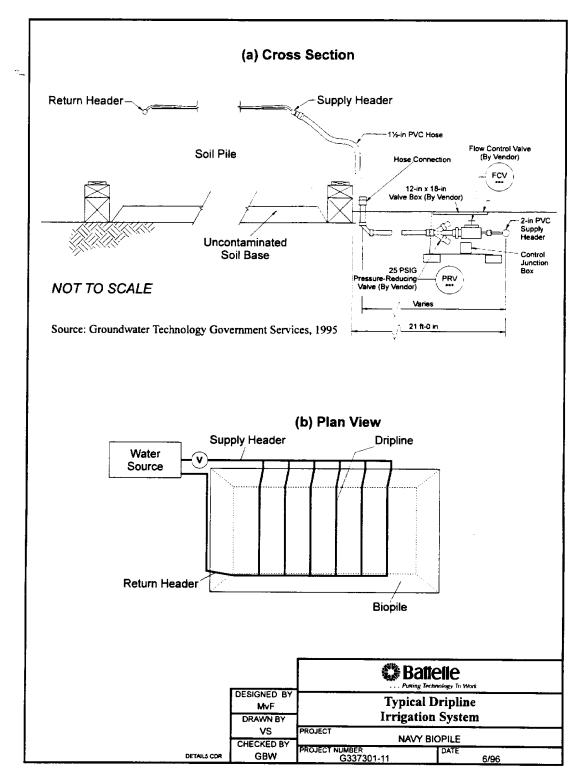


Figure 19. Typical Dripline Irrigation System

The dripline irrigation system has three disadvantages:

- The initial cost is a factor not required by other types of irrigation.
- Some installation time is required.
- The dripline may break and spring a leak to cause excessleachate.

**6.1.6 Leachate Collection System.** The biopile configuration featured in this document is designed to minimize the formation ofleachate. Therefore, unless mandated by special circumstances or regulations, the construction of a leachate collection system should not be necessary, because the theorem liner is designed to contain any water or leachate that migrates to the bottom of the pile. However, if aleachate collection system is warranted, it should be constructed by sloping thebiopile base toward one corner of the pile to channel anyleachate to a leachate collection pipe or preferably by incorporating theleachate collection with the aeration system, as shown in Figure 3.

If a separate collection pipe is used, it should be a 2-inch (5.1-cm)-diameter slotted PVC pipe connected to a blank 2-inch (5.1-cm)-diameter PVC pipe that leads to aleachate collection sump or tank. If aleachate collection sump is used, it must be located below the grade of thebiopile foundation and must be lined with an impermeable layer, such as a plastic tank.

If an aboveground leachate collection tank is used, it should be located within a secondary containment area, such as a lined berm and foundation that are large enough to contain the volume of the tank. A pump would be required to transfer the leachate from the collection pipe to the aboveground storage tank. The tank size can range from 500 to 1,500 gal (1,895to 5,685 L).

One example where aleachate collection system may be warranted is in locations having strong winds. An exceptionally strong or persistent wind potentially may damage theoiopile cover so that rainwater could saturate the pile and leach through it. If aleachate collection pump is used, a liquid level controller (LLC), such as a float switch, may be useful to turn the pump on and off asleachate collects and is removed from theleachate collection point. When a leachate collection system and abiopile irrigation system are both installed, theleachate can be recycled back to the pile through the irrigation lines.

## 6.1.7 Biopile Formation.

**6.1.7.1 Soil Preparation.** Contaminated soil is either loaded onto thebiopile pad or preprocessed prior to forming the biopile. The initial soil characterization data that are required for selecting thebiopile technology provides information for identifying the need for soil blending or shredding. These data include the particle size, clay and silt content, and void volume. The soil preprocessing can involve screening bulk objects, such as large rocks, followed by soil shredding and/or soil blending with a bulking agent. The extent of preprocessing required is a function of the soil type, soilcomposi-tion, and contamination level. For example, soils with low soil porosity (void volume <25%) could inhibit the airflow through the pile. Adding a bulking agent, such as rice hulls, shredded cardboard, or wood chips, would increase the void spaces in the amended soil matrix and thus improve the airflow through the pile. Generally, the soil needs to be screened prior to the soil shredding or blending step, to prevent damage to the shredding and mixing equipment. Typical soil processing equipment includes:

- bucket loader and/orskidsteer loader
- parallel bar screen to remove large, heavy objects
- trommel screen or vibrating screen for finer particle screening
- soil shredder
- a soil mixer, such as a KnightReelAuggie<sup>TM</sup>, with a load cell for weighing and blending soil and amendments (including nutrients, bulking agent, and water).

Soil screening, shredding, and mixing equipment often can be rented or operated by a contrator. Soils not requiring shredding or blending may be applied to the biopile without a mechanical screening step. Some hand removal of large rocks or debris may be required in any case. For soil movement without screening or blending, the only equipment required is the bucket loader.

For cases where a soil mixer is employed, water and nutrients can be introduced into the mixer along with the soil. Nutrient addition is covered in Section 6.1.4, and Section 6.1.5 covers moisture addition.

**6.1.7.2 Pile Formation.** Following any soil preparation, the loader places soil on theoiopile pad. The pile is formed by applying the soil in rows by working from "back to front," starting at one side and moving to the other side as each row of soil fills the pad to the desired height of 5 to &t (1.5 to 2.4 m). The "back" is the side where the aeration header is located. The "back" is also the side of the cell base that is opposite the side the loader enters. The term "front" refers to the side where the loader enters theoiopile pad. The front-end loader must never be allowed to drive over previous lifts.

The loader must be careful not to drive over the aeration pipes as the pile is being formed. Also, to prevent damage to the containmentberm, the side opposite the aeration manifold header can be removed to allow the loader to drive directly onto the clean soil foundation without driving over theorem. As an alternative, the loader can form a soil ramp to bridge theberm. For a permanentbiopile cell, a ramp might be built. To form a relatively smooth heap, the loader operator should fill in and even out the rut between the rows as the pile is being constructed. The pile should have a relatively smooth top and a slight grade from one side to the other to prevent pooling of rainwater on the pile cover. Advising the loader operator in advance of the required configuration of the pile top will help to ensure the proper pile top finish.

When soil is applied to the biopile pad, have the loader operator leave a 1- to 2-ft (0.3 to 0.6-m)-wide space between the soil pile and the containmentberm. This space will prevent soil from falling over the berm as the final pile height is reached, the pile top is graded by the loader, and water is added to the pile.

After the soil has been applied, the side of theberm removed earlier can be reinstalled or the soil ramp can be removed and added to the pile. The targetbiopile dimensions for a 500-yd (382-m<sup>3</sup>) system are 50 ft  $\times$  60 ft  $\times$  5 to 8 ft high (15.2 m  $\times$  18.3 m  $\times$  1.5 to 2.4 m high), with a pile slope of 1.25:1 side-to-height ratio. Figure 16 depicts the constructed pile dimensions.

**6.1.7.3 Installing Soil Gas Monitoring Points** Error! Bookmark not defined. Soil gas monitoring points are used to sample the biopile soil gas and can be installed as the pile is being formed. Data collected from the soil gas include oxygen, carbon dioxide, and TPH concentrations. A typical monitoring point construction is illustrated in Figure 20. Table 4 gives the monitoring point parts list and potential vendor sources.

Part	Supplier	Part #
1/4-inch nylon tubing	Cole-Parmer	H-96141-44
NPT 3/8×1/4 tube fitting	NewAge Industries	5201098
female quick-connect	Forberg Scientific	4Z-Q4CN-BBP
gravel for strainer	any aquarium supply	none
3/8 inch thread tap	any hardware store	none
suction strainer 3/4 inch	Grainger	2P052
male quick-connect×1/4 inch tube	Forberg Scientific	4Z(A)-Q4P-BBP
mini-male thermocouple plug	L H Marshall	3060-K
thermocouple wire	L H Marshall	K24-1-508
quick-connect protectors	Forberg Scientific	CP-Q4C-BB

The monitoring points can be connected to the ¼-inch (0.64-cm)-diameter nylon or polythylene Table 4. Monitoring Point Parts List

sampling tube and held in place as the loader empties a bucket of soil over the monitoring point. The desired depth of the monitoring point should be approximately 3 to 4 ft (0.9to 1.2 m) below the pile top and a minimum of 1 ft (0.3m) above the uncontaminated soil (Figure 21). Although placement of the monitoring points is at the project manager's discretion, they should be installed to measure activity in various pile locations, such as at the edge, in the middle, and over the aeration pipe versus in between the pipes. The ¼-inch (0.64-cm)-diameter sampling tube should be long enough to extend 5 ft (1.5m) beyond the containmentberm of the side nearest the point. Having the tube extend beyond the containmentberm will enable easy access to the monitoring points without having to cut through or walk on the plastic pile cover.

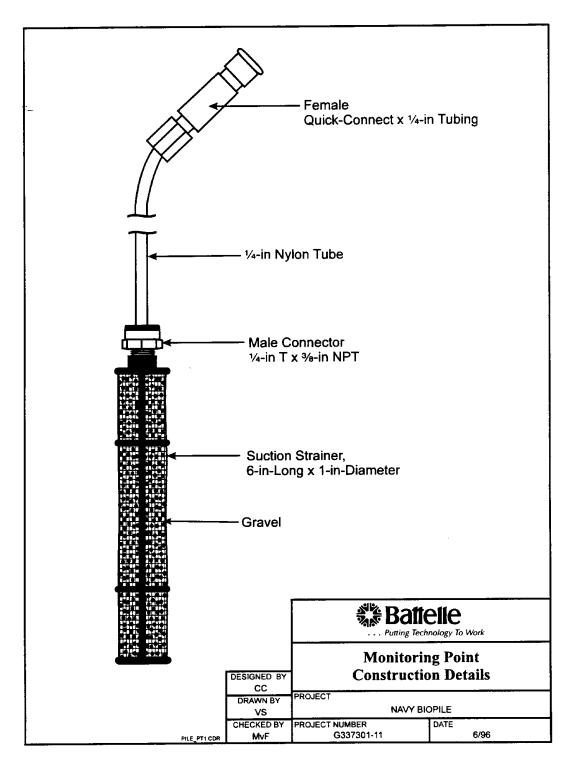


Figure 20. Typical Monitoring Point Construction

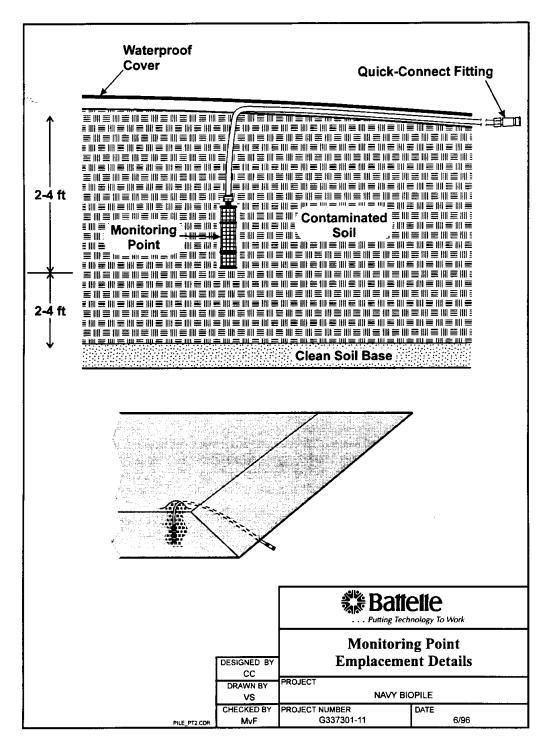


Figure 21. Suggested Monitoring Point Emplacement Technique.

The location of the soil gas monitoring points should be recorded, and all monitoring points should be labeled for data-recording purposes. The number of monitoring points to be installed will vary with the data collection requirements. Generally, 6 to 8 monitoring points will be sufficient for each 500-ytd(382-m<sup>3</sup>) biopile.

To track soil temperature in the biopile, thermocouples or thermistors can be installed with the monitoring points. As with the soil gas sampling tubes, the thermocouple wire should be long enough to extend beyond the berm of the pad. If desired, moisture sensors can be installed to track the moisture content of the soil.

**6.1.7.4 Time-Zero Sampling.** Sampling the biopile upon completion of construction establishes initialbiopile conditions that will serve as the reference point for the TPH degradation progress, microbial activity, and moisture variations over the treatment period. Two types of samples are required: soil samples and soil gas samples.

Soil samples are collected using a hand auger as described in Section 6.3. As a minimum guideline, one soil sample should be collected for every 100yd<sup>3</sup> (76.5 m<sup>3</sup>). A sampling frequency of 1 sample per 50 yd (38.2 m<sup>3</sup>) would be ideal if the contamination and soil property distributions are not known to be homogeneous. Soil samples are then transported to a laboratory and analyzed to establish the initial TPH concentration, BTEX concentration, moisture content, pH, microbial density in soil, and toxic metals concentration (if not already known).

Soil gas samples are taken by pulling a gas sample from the monitoring points installed during the biopile construction. Soil gas readings of Q,  $CO_2$ , and TPH concentrations can be taken by pulling the sample directly through soil gas detectors: one detector for Q and  $CO_2$  and a second one for TPH measurement. For off-line sampling, soil gas can be pulled through a vacuum pump to aTedlar<sup>TM</sup> sample bag. The gas sample can then be transported to a laboratory for analysis.

Soil gas sampling can establish the effectiveness of the aeration system and can be used to establish the microbial activity via a respiration test. After placing the system into operation, the Qconcentration profile of the soil gas at the different monitoring points is an indicator of the aeration effectiveness. Relatively consistent Q concentrations point to even aeration of the pile, which is desirable. In addition to aeration, a second point to consider is sufficient airflow rate. Measurements of Q concentration in the soil gas of 15 to 21% indicate the airflow rate is sufficient.

The respiration test is performed to establish the microbial activity in the soil that can be related to the TPH degradation rate. The respiration test is conducted by turning off the aeration pump and measuring the decrease in  $O_2$  concentration and increase in CQ concentration with time. The more rapidly the Q concentration decreases, the more active the microbial population. Because aerobic microorganisms require Q to degrade TPH, the amount of Q consumed per time (consumption rate) can be related to the TPH degradation rate by using hexane as the representative TPH compound. Equation 1 shows the relationship between Q consumption and TPH degradation.

$$C_6H_{14} + 9\frac{1}{2}O_2 \rightarrow 6CO_2 + 7H_2O \tag{1}$$

Appendix I gives a step-by-step method for calculating the TPH degradation rate from respiration test data.

The quantity, depths, and locations of the soil samples and monitoring points should be recorded into the site record book. The locations can be fixed by using a grid coordinate system with one corner of the containment berm serving as the (0,0) coordinate reference point. A 75- or 100-ft (22.9 or 30.0-m) surveyor's measuring tape can be used to establish the soil sample and the montoring point coordinate locations. Soil gas data should be recorded as it is collected.

**6.1.7.5 Cover Installation.** After the soil samples have been taken, the cover can be installed. The cover should be a waterproof plastic, such as HDPE, and should be  $75 \times 75$  ft ( $22.9 \times 22.9$  m) in area. The thickness, approximately 12 to 20 mil, should be sufficient to make the cover resistant

to stretching or tearing. The cover should be black or some other opaque color. White is least preferred because it reflects sunlight radiation the most.

Prior to covering the biopile, ensure that the monitoring point tubes extend beyond the containment berm, because they will need to be sandwiched between the containmenberm and the cover to allow easy access for future soil gas sampling. Additionally, remove any sharp objects, such as wire, rebar, or jagged rocks, from the iopile top to prevent ripping the cover during installation. Ensure there are no low points in the center of the biopile that could serve as water collection areas. The cover should first be unfolded and inspected for any significant holes or tears, because any holes can allow rainwater to seep through the pile and cause deachate problem. Pulling the cover over the biopile will require a minimum of 2 people. However, 3 or 4 persons would be preferable, because a tarp of that size will be heavy.

Once the cover has been pulled over the biopile and extends over the containmentberm on all four sides, it can be secured by sandwiching it between the woodberm and 6-ft (1.8-m) sections of 2-inch×4-inch (5-cm×10-cm) beams. The  $2\times4s$  can be attached using 3-inch (7.6-cm)-long 5/16-inch (0.8-cm) lag screws. The screws should have a hex head to allow them to be driven with a drill. Leaving a 2- to 8-inch (5 to 20-cm) spacing between beams will create runoff channels for rainwater. The cover should be pulled snugly over the pile as the beams are installed. A snug cover will be less likely to rip during high winds than a loose cover.

Once the  $2 \times 4s$  have been attached to the cover andberm, nylon ropes can be used to tie the cover across the surface of the pile. To attach the ropes, drill three eye-screws to each side of the containmemberm, placing one in the middle of each side and spacing the remaining two eye-screws an even distance to each side of the center. The nylon rope can be run through the eye-screws and over the pile in a "V" fashion as shown in Figure 22.

**6.2 Permanent Biopile Design.** The size and cover for a permanentbiopile are the same as for the temporary system. The overall facility space requirements may be larger than for the tem**pra**ry biopile system, because additional support facilities and space for future expansion may be desired. When planning a permanent biopile site, the following questions should be asked:

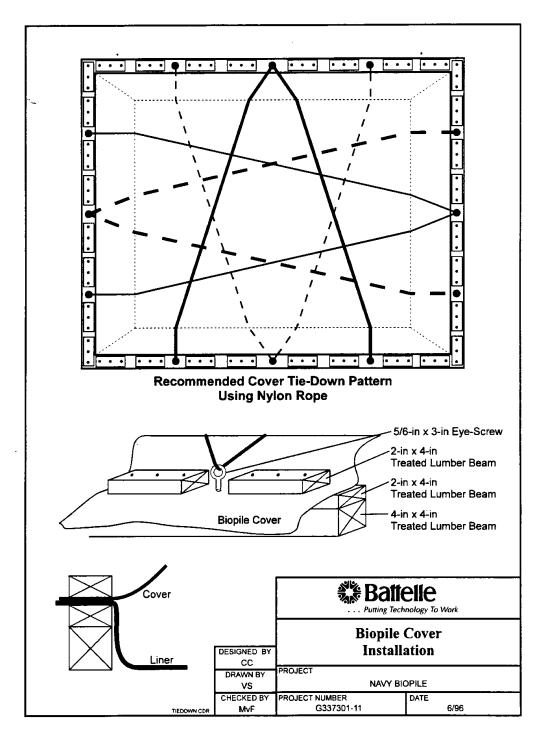


Figure 22. Suggested Pattern for Tying DownBiopile Cover with Nylon Rope.

• How much soil volume will be processed per month? The quantity will be used to calculate the number of biopile modules needed.

- What additional support features would be desirable?
- What, if any, possible need will there be for future expansions?

Any features a permanent site should have beyond the emplacement of the permanenbiopile design will be site-specific. This section focuses on generally uniform features of permanenbiopile sites, i.e., the design of the permanent biopile pad, the aeration system, theleachate collection system, the moisture addition system, and nutrient addition. Permanentbiopile sites also may have upgraded support facilities, such as an equipment storage building, a designated equipment decontamination pad, and a building to serve as an office and control center. Other potential features would be a truck scale to track the amount of soil arriving and leaving the site and improved access roads and materials-handling areas. The extent to which facilities are upgraded is a function of site-specific needs and available funding.

**6.2.1 Concrete Biopile Pad.** The main difference between the temporary and permanenbiopile designs is in the pad construction. The permanentbiopile design will have a concrete pad that has an area of 50 ft× 60 ft ( $15.2 \text{ m} \times 18.3 \text{ m}$ ). The thickness of the concrete will be specified by the applicable construction guidelines. Figure 23 illustrates the general construction format of the permanentbiopile cell. The example given in Figure 23 shows a 7-inch (17.8-cm)-thick concrete pad installed on an 18-inch (45.7cm)-thick compacted fill foundation.

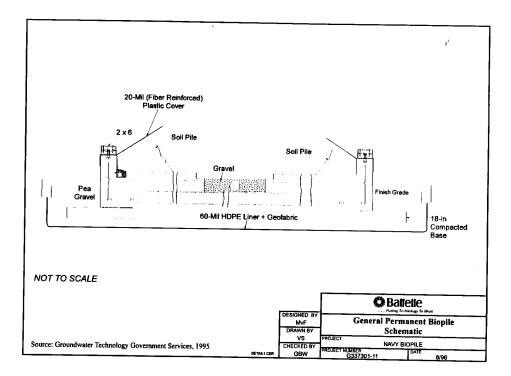


Figure 23. General ConcreteBiopile Schematic.

Whereas the temporary biopile pad liner is a 60-mil HDPE liner lying on the compacted base, the permanent pad liner should be composed of a 60-mil HDPE sheet laid on æcosynthetic liner emplaced directly beneath the compacted fill.

Unlike the temporary biopile, the aeration legs will be emplaced below grade in concrete channels and covered with pea gravel or similar material. The leachate containment berm should be approximately 6 inches (15.2 cm) high, constructed from concrete, and topped with a 3-inch × 3-inch (7.6-cm × 7.6-cm) pressure-treated wood

nail rail attached to three sides of theberm. The fourth side, the one opposite the aeration line influent points, will have a 6-inch  $\times$  2-inch nail rail anchored to the top of theberm, because that is the side from which the loader will access the biopile. This nail rail will be used to anchor the cover to thebiopile pad in a similar way as done on the temporary pile. Additionally, to enable easy access by the loader, a concrete ramp should be constructed on both sides of the berm on the side of the pad where the loader will enter to add contaminated soil. Figure 24 illustrates the construction of the biopile side that will serve as the loader access side. The actual pad design may vary, depending on the site conditions and client requirements.

**6.2.2 Aeration Syst em**. Generally, there will be three aeration legs and therefore three channels in the concrete pad. Unlike the temporarybiopile, the aeration legs will beemplaced below grade in 6-1/2-inch (16.5-cm) deep, 7-inch (17.8-cm) wide concrete channels and covered with pea gravel or similar material (Figure 25a). The aeration legs should be constructed of 2-inch (5.1-cm) diameter schedule 80 PVC pipe. The first 10 ft (3 m) of each aeration leg should be smooth pipe. The remaining 40 ft (12.2 m) should be slotted pipe. Where the aeration legs intersect, the concrete containmentberm, the piping should be 2-inch (5.1-cm) diameter galvanized steel to reduce the risk of having to cut through the concreteberm to replace a cracked pipe (Figure 25b). The actual depth will be specified by the contractor or base engineer installing the pad. The channels will be approximately 40 ft (12.2 m) long, because the aeration pipe will only extend 40 ft (12.2 m) into the pile, to prevent short-circuiting of the air flowing into the pile from the ambient atmosphere.

The aeration leg channels can be connected to aleachate collection channel leading to theleachate collection tank. Figure 25 illustrates the aeration channels. Asleachate generally will collect in the aeration channels, the channels should have a 1- to 2-degree slope toward the aeration channel that parallels the aeration header pipe and leads to theleachate collection sump. If the aeration system is operated in the extraction mode, the leachate collection system can be incorporated into the aeration system, as shown in Figure 3.

The aeration system could be automated by using either an on-off timer switch or a variable-speed vacuum pump controlled by a feedback loop with an Q sensor. The Q sensor would pull soil gas samples from several locations in the biopile, average the readings, and use a rule-based control routine to adjust the vacuum pump speed. Over aeration leads to premature drying of the soil. Using a simple process control loop to regulate the airflow rate by measuring the Q concentration in

the pile will eliminate over aeration of thebiopile and may reduce or eliminate the need to replace moisture during a biopile batch run.

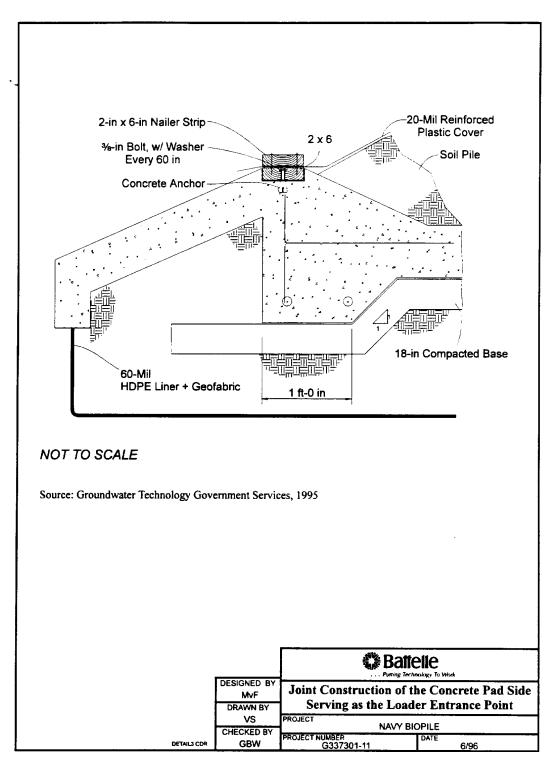


Figure 24. Joint Construction of the Concrete Pad Side Serving as the Loader Entrance Point.

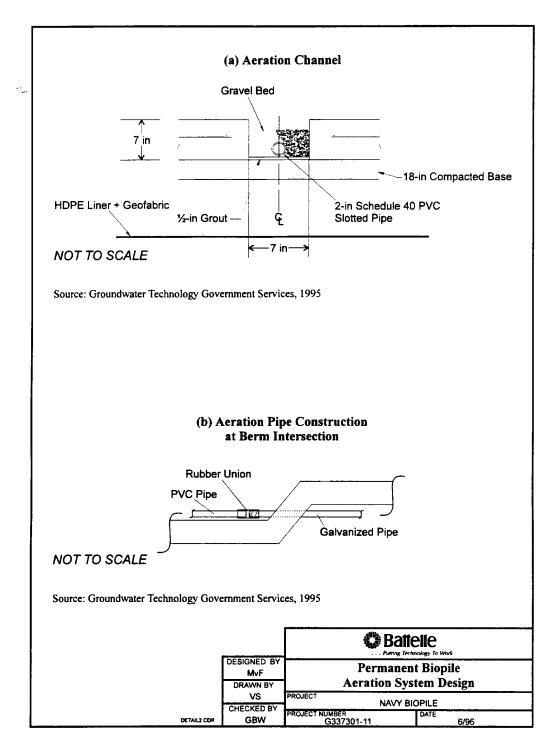


Figure 25. PermanentBiopile Aeration System Design: (a) aeration channel; (b) aeration pipe construction atberm intersection.

As specified in Section 6.1.3, the air should be pulled through thebiopile using either a 1½-hp (1,120W) regenerative centrifugal blower (preferred) or a rotary-positive displacement blower capable of drawing approximately 120scfm. The overall aeration system is described in Section 6.1.3.

**6.2.3 Leachate Collection System** The leachate collection system will consist of the aeration pipe channels and a 4-inch to 6-inch (10.2-cm to 15.3-cm) leachate collection channel running parallel to the aeration header pipe inside of the containment berm. The collection channel should have a 1 to 2-degree slope toward one end of the channel where a 2-inch (5.1-cm)-diameter slotted PVC leachate collection pipe is located. The collection pipe is joined to a 2-inch (5.1-cm) galvanized steel union that traverses the concreteberm and reconnects to a 2-inch (5.1-cm) PVC transfer line leading to a \_-hp transfer pump that pulls theleachate to the leachate collection tank. The pump can be activated by a liquid level controller (LLC) installed in the slotted PVCleachate collection pipe. The system can be set up so that stored leachate is recycled back to thebiopile via the automated irrigation system described in Section 6.2.4. Alternatively, the leachate collection system may be incorporated with the aeration system, as shown in Figure 3.

**6.2.4 Automated Irrigation System** . An irrigation system is generally not needed but may be required in dry climates. Figure 26 shows a typical installed automated irrigation system. The system would consist of a flexible dripline or soaker hose that would be placed over thebiopile in three or four discrete strips joined by a common feed line that would feed between the containmentberm and the cover. Although not yet evaluated by NFESC, the irrigation system could be automated by placing moisture sensors in thebiopile that would activate a water valve when the soil moisture content drops below a certain humidity. As an alternative, the irrigation line could be operated manually or via a timer switch.

**6.2.5** Nutrient Addition. Nutrients can be added in several ways. Where possible, nutrients should be added as the biopile is being formed, by mixing nutrients in with the soil as it is being processed, by spraying a nutrient solution of known concentration on the soil as it is being placed on the biopile cell, or by adding dry nutrients with each bucket of soil. Table 5 indicates the composition of several agricultural chemicals that can be used to adjust the nutrient content of soils being prepared forbiopile treatment.

Name	Formula	Molecular Weight	Percent N:P:K	Vendor Quoted
Urea	CO(NH <sub>2</sub> ) <sub>2</sub>	60.03	46:0:0	Lesco
Super phosphate	$Ca(H_2PO_4)_2$	234	0:27:0	Lockbourne Farmers' Exchange Co.
Potassium sulfate	$K_2SO_4$	174	0:0:45	Lesco
Diammonium phosphate	(NH <sub>3</sub> ) <sub>2</sub> PO <sub>4</sub>	129	22:24:0	Lesco

Table 5. Types of Nutrient Sources

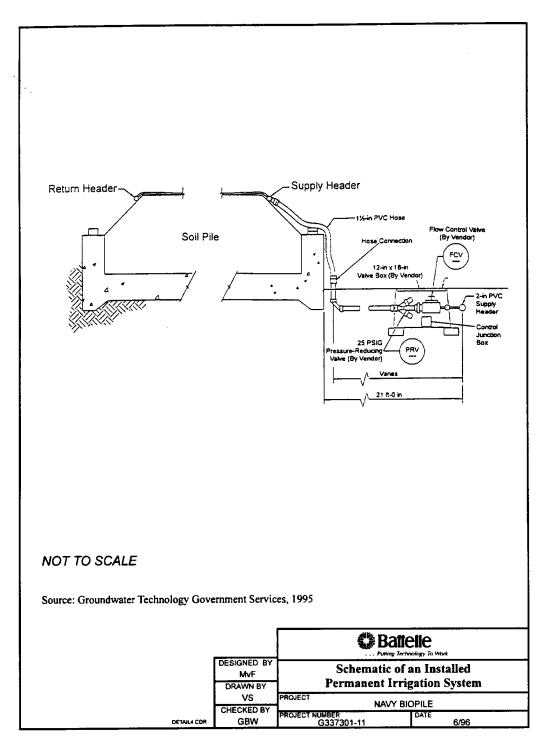


Figure 26. Schematic of an Installed Irrigation System

The irrigation system could double as a nutrient addition system by dissolving the nutrients and adding them in liquid form. Using the irrigation system to add nutrients would be especially useful in cases where batches

require longer processing times, more than 4 months for example, or where it is seen as advantageous to add nutrients over the course of the process. One example of the latter case would be where **a** cometabolite, such as a supplemental carbon source, would be required to process a recalcitrant contaminant.

**6.2.6 Soil Processing.** The need to screen, shred, and blend soils will vary from site to site. For a permanent biopile facility, soil processing equipment requirements will be a function of the expected soil type and contamination level. The need to improve the soil condition should be weighed against the additional cost of leasing or purchasing soil-processing equipment.

Generally, soils with TPH contamination in excess of 50,000 mg/kg will require blending with uncontaminated or less-contaminated soil or with an organic amendment. Soils high in clay or silt content, or that are otherwise airflow limiting, will require shredding and blending with a bulking agent, such as rice hulls, wood chips, or shredded cardboard. Airflow limiting is defined in this

document has having a soil void volume of less than 25%. Some highly contaminated soils pack tightly together to form large, sticky clumps. Air cannot effectively penetrate such clods. As a result, the microbial degradation rate and final achievable reduction in contamination will decline.

Shredding the soil and, as necessary, blending it with a bulking agent will incre**se** the surface-to-volume ratio of the soil particles, thereby increasing the amount of contamination exposed to *Q* and the subsequent microbial degradation. Prior to feeding soil to a shredder, a screening step

will be required to remove large rocks and foreign objects, such as metal rods and tires. Screening foreign objects from the soil will protect the shredder hardware and reduce the risk of projectiles shooting from the shredder. Upon being shredded, the soil can be fed to a soil mixer, such as a paddle mixer. This mixer can be truck-mounted or stationary and can be equipped with a load cell to measure the amount of soil, amendments, and water being fed. The mixer can serve to blend in nutrients and to add the required amount of moisture. Having a soil mixer provides the site operators with an ideal situation with respect to soil preparation. From the mixer, a loader can transport the processed soil to the biopile for treatment. Figure 27 illustrates a typical soil processing train.

**6.3 Sampling and Analysis.** This section outlines the sampling and analysis methods required fobiopile design and routine monitoring activities. The procedures for soil, soil gas, and eachate collection are described in detail in this section, as well. Table 6 outlines the testing parameters for each matrix and lists the associated analytical methods for each test.

**6.3.1 Soil Sampling Methods.** A variety of methods are available to collect soil samples; however, the handauger sampling method is the best suited for sampling abiopile. Use the hand auger to bore a hole to approximately 1 ft (0.3 m) above the desired sampling depth. Next, use a slide hammer-type hand sampler, lined with brass sleeves (two 6-inch sections) to collect a core sample. Remove, cap, and label the two brass sleeves containing the soil sample. Upon labeling the samples,

complete chain-of-custody form and place the sample in a cooler chilled with artificial ice. Upon completion of sampling, transport samples to a test lab.

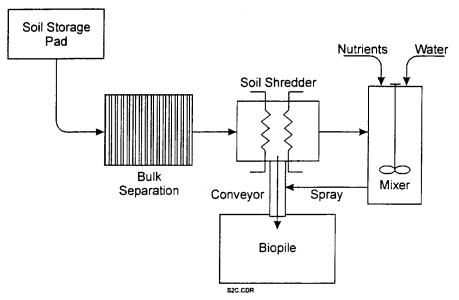


Figure 27. Typical Soil Processing Flowchart

**6.3.2.1** Tedlar<sup>TM</sup> Sampling Bag Method. A Tedlar<sup>TM</sup> gas sampling bag is used to collect soil gas samples for field measurements. The soil gas samples are obtained from the soil gas monitoring points. Soil gas can be either pumped into the sampling bag using an inert pump or pulled into it by connecting the bag directly to the sample line and then placing the bag in a portable vacuum chamber. The soil gas sample will be analyzed on site using hand-held gas analyzers for Q<sub>2</sub>, CO<sub>2</sub>, and TPH concentrations. Sampling by this technique is easy to perform, but care must be taken not to cross-contaminate samples through improper cleaning of the bags between samples. Before reusing the Tedlar<sup>TM</sup> bag, ensure that it is flushed twice with ambient air and once with soil gas.

Soil temperature data are collected by means of soil temperature thermocouples othermistors placed in predetermined locations and depths duringbiopile construction. The data are collected through the temperature sensor lead located in the monitoring probe box. Temperature readouts are obtained by connecting a Fluke 52 or equivalent digital readout instrument to the temperature sensor lead and recording the digital readout. Soil gas in the biopile is sampled accourding to the following procedure:

- Step 1. Connect the vacuum pump to the quick-connect coupling at the monitoring probe. A trap can be placed between the pump and the monitoring probe to collect any water that might be pulled from the monitoring probe.
- Step 2. Connect the pump outflow to a 1-LTedlar<sup>™</sup> bag.

- Step 3. Turn on the pump and fill the Tedlar<sup>™</sup> bag with soil gas, making sure that the bag valve is in the open position.
- Step 4. Flush the bag with soil gas twice using steps 1 through 3 and collect the final soil gas sample.
- Step 5. Close the valve on the bag and disconnect the bag from the pump.
- Step 6. Analyze the soil gas in the bag for oxygen, carbon dioxide, and TPH using a portable gas analyzer. Record the readings.

**6.3.2.2 Direct Soil Gas Sampling Method.** A convenient alternative to the Tedlar<sup>TM</sup> bag sampling method is to directly attach the gas detectors to the monitoring point lines via the quick-connect couplings.

- Step 1. Calibrate the  $O_2/CO_2$  detector and the TPH detector using the appropriate span gases.
- Step 2. Attach the  $O_2/CO_2$  detector to the monitoring point and draw soil gas through the detector until the reading stabilizes. Do not record the initial reading, because enough soil gas must clear through the detector to equal the amount originally in the monitoring point sampling line.
- Step 3. Record the  $O_2$  and  $CO_2$  readings in the data record book.
- Step 4. Repeat steps 1 through 3 using the TPH meter.

**6.3.3 Leachate Sampling Method.** Leachate samples should be collected by a grab sampling method. Grab samples of surface water are collected manually in a clean glass vessel and transferred immediately to a volatile organic analysis (VOA) vial. ATeflon<sup>TM</sup>-lined cap is installed and the vial is inverted to ensure that there is zero headspace. Multiple vials may be filled from the single grab sample so that enough water is available for the analyses. After the sample is contained and properly labeled, it is shipped to a laboratory for the appropriate analyses.

Matrix	Characteristic	Analysis	Method	Method Objective Determines:
Soil	Physical	Soil classification	ASTM D 2488	The soil type as defined by the Unified Soil Classification System based on visual observations
		Particle-size distribution	ASTM D 422	Particle-size distribution by sieve size on a dry soil sample
		Bulk density	ASTM D 4532	The weight per unit volume of an oven-dried soil sample.
		Porosity	ASTM D 2434	The pore space in a soil sample by dividing its bulk density by the particle density.
		Moisture content	ASTM D 2216	The weight percent free water by oven-drying at 110°C.
		рН	EPA SW-846 Method 9045	pH of soil or waste by mixing the sample with reagent water and measuring the aqueous solution with a pH meter.
	Contamination	Total petroleum hydrocarbon oils	EPA SW-846 EPA Method 503	Oil and grease by a liquid/liquid extraction to collect organics for analysis by evaporation and gravimetric methods. Measures any of the heavier hydrocarbons includ-ing asphaltic materials and materials of biogenic origin.
	Nutrients	Total Kjeldahl nitrogen	EPA Method 351.4	Nitrogen through digestion of the sample followed by titration with the measurement of ammonia generation using a potentiometric or ion-selective electrode.
		Phosphorous, all forms	EPA Method 365.2	Specific forms of phosphorous through the reaction of the sample with reagents that generate complexes that are reduced by ascorbic acid. Colorimetric measurement is made using a spectrophotometer.
		Potassium	EPA Method 200.7/ 6010	Potassium through an acid digestion followed by elemental analysis by inductively coupled atomic plasma (ICAP) emission spectrometric method.
	Biological	Hydrocarbon degrader density	Standard Method 9215B	The indigenous microbial enumeration of hydrocarbon degraders through the inoculation of noble agar with local hydrocarbon contamination as the only available substrate for organisms isolated from the sample.
Soil Gas	Contaminant	Total petroleum hydrocarbon (TPH)	EPA Compendium Methods TO-3 (modified)	TPH concentrations in whole-air soil gas sample through cryofocused sample preconcentration followed by a gas chromatography/flame ionization/photoionization detection technique.
	Biological (Respiration)	Oxygen	Monitored with a GasTech instrument	The percent oxygen concentration in a whole-air soil gas sample.
		Carbon dioxide	Monitored with a GasTech instrument	The percent carbon dioxide concentration in a whole-air gas sample.
Leachate	Contaminant	TPH (oils)	EPA SW-846 Method 3510 EPA Method 503	Oil and grease content by a liquid/liquid extraction method to collect organics for analysis by evaporation and gravimetric methods. Measures any of the heavier hydrocarbon including asphaltic materials and materials of biogenic origin.

# Table 6. Summary of Analytical Methods forBiopile Design

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### APPENDIX A

## GLOSSARY OF TERMS USED TO DESCRIBE BIOPILE DESIGN AND IMPLEMENTATION

acidity - measure of the hydrogen ion concentration of a solution

adsorption - the process by which molecules collect on and adhere to the surface of an adsorbent solid due to chemical or physical forces

aeration - process of supplying or introducing air into a medium such as soil or water

aerobic biodegradation - biodegradation occurring in the presence of oxygen

alkalinity - measure of the hydroxide ion concentration of a solution

ARAR - Applicable or Relevant and Appropriate Requirement

aromatic hydrocarbon - a class of hydrocarbons consisting of cyclic conjugate carbon atoms, e.g. benzene

asphaltene - a dark, solid constituent of petroleum that is soluble in carbon disulfide but not soluble in paraffin naphthas

BDAT - best demonstrated available technology

biodegradable - a material or compound that is able to be broken down by natural processes of living things such as metabolization by microorganisms

biodegrade - breaking down material (usually into more innocuous forms) by natural processes of living things such as metabolization by microorganisms

biodegradation rate - the mass of contaminant metabolized by microorganisms per unit time. In soil contamination this is normalized to the mass of soil and usually is expressed as mg contaminant degraded/kg soil-day (mg/kg-day).

biopile - soil pile constructed to allow aerobicbioremediation by aeration, possibly supplemented with water and nutrient additions

bioreactor - a container or area in which a biological reaction or biological activity takes place

bioremediation - general term for the technology of using biological processes such as microbial metabolism to degrade soil and water contaminants and decontaminate sites

bioventing - an in situ soil aeration process designed and operated to maximize the iodegradation of organic compounds, with some volatilization occurring

blower - a unit of rotating mechanical equipment used to increase the pressure in a gas stream and providing a total pressure rise of more than 4 inches of water and less than 14.7 psi

BTEX - benzene, toluene, ethylbenzene, and xylenes

bulking agent - biodegradable organic material, such as rice hulls or wood chips, added to improve the permeability, water-holding capacity, or other properties of soil to be treated in biopile

C - carbon

CAA - Clean Air Act

CFR - Code of Federal Regulations

CFU - colony-forming unit. Measuring the number of CFUs is a low-cost screening method to determine the ability of a contaminated matrix to sustain microbial action.

clay - fine-grained soil that can exhibit putty-like properties within a range of water content and which shows considerable strength when air-dry

contaminant - something that makes material in contact with it impure, unfit, or unsafe; a pollutant

CWA - Clean Water Act

cycloalkene - unsaturated, monocyclic hydrocarbon with the formula  $GH_{2n-2}$ 

DC - direct current

DOD - U.S. Department of Defense

ex situ - refers to a technology or process for which contaminated material must be removed from the site of contamination for treatment

extraction - aerating the biopile by removing air under vacuum to induce airflow

FC - field capacity

FID - flame ionization detector

field capacity - the amount of water held in soil after excess water has drained away and after the rate of downward drainage has become negligible

free product - organic contaminant existing as a separate liquid phase

GAC - granular activated carbon

GC - gas chromatograph

hand-auger drilling - hand-drilling by rotating a spiral channel supported on a shaft

HC - hydrocarbon

HDPE - high-density polyethylene

head - the pressure difference between two places, an energy term expressed in length units

Henry's law constant - the partial pressure exerted by a compound divided by the concentration of the compound in aqueous solution. The Henry's law constant of low-solubility compounds can be approximated as the ratio of the pure component vapor pressure and the water solubility.

heterotrophic bacteria - bacteria that obtain energy and carbon from organic molecules

ICAP - inductively coupled atomic plasma

ICE - internal combustion engine

impermeable membrane - sheeting material designed to retain water

injection - aerating the biopile by forcing in air under vacuum to induce airflow

in situ remediation - a treatment process that can be carried out within the site of contamination without bulk excavation

JP - jet propulsion (fuel)

K - elemental potassium

LDR - Land Disposal Restriction

leachate collection point - sand-filled area to whichbiopile leachate drains and from whichleachate is transferred to the collection tank via a leachate suction line

LLC - liquid level controller

LNAPL - light, nonaqueous-phase liquid

MCL - maximum contaminant level

MCLG - maximum contaminant level goal

mineralization - the complete conversion of an organic compound to inorganic products (principally water and carbon dioxide)

monitoring point - soil gas sampling port consisting of a porous gas collection port connected to tubing that is placed in the biopile to allow withdrawal of a gas sample for analysis

N - elemental nitrogen

NAAQS - National Ambient Air Quality Standards

NFESC - Naval Facilities Engineering Service Center

NPDES - National Pollutant Discharge Elimination System

nutrient amendment - chemical or organic fertilizer, usually rich in nitrogen, phosphorus, or potassium, that is added to support life and growth of microorganisms in thebiopile

O<sub>2</sub> - molecular oxygen

off-gas - gaseous effluent, possibly containing contaminant vapors, that leaves a process, typically from a point source during process operations

OSHA - Occupational Safety and Health Administration; Occupational Safety and Health Act

oxygen use rate - rate of oxygen consumption due to biological and chemical action (used to determine respiration rate when the chemical oxygen demand is negligible)

permeability - measure of the capacity of a rock, soil, or sediment to allow passage of liquid or gas through pores without damage to the structure of the media

PAH - polycyclic aromatic hydrocarbon

PCB - polychlorinated biphenyl

pH - measure of the acidity or alkalinity of a solution; the negative log of the hydrogen ion activity

PID - photoionization detector

pipe schedule - standard method for designating the wall thickness of pipe

pore space - the open space and minute passages in a solid material

porosity - measure of the amount of available pore space in a material through which liquid and gas can move

POTW - publicly owned treatment works

ppmv - part(s) per million by volume (indicates vapor concentration)

PVC - polyvinyl chloride

RA - remedial action

RCRA - Resource Conservation and Recovery Act

remediation - activity involved with reducing the risk from a contaminated site

respiration - oxidation of compounds to provide energy for cells

respiration rate - rate of reduction of oxygen concentration due to biological action

respiration test - test used to provide rapid field measurement obiodegradation rates to determine the potential applicability of aerobic bioremediation at a contaminated site and to provide information for a fullscale treatment system design

rH - relative humidity

sand - unconsolidated rock and mineral particles with diameters ranging from 1/16 to 2 mm

scfm - standard cubic feet per minute

shredding - mixing and grinding soil to improve homogeneity and increase permeability

short-circuiting - undesirable condition in which air flows unevenly through the iopile due to the existence of low-resistance pathways

silt - unconsolidated rock and mineral particles with diameters ranging from 0.0002 to 0.05 mm

sLm - standard liter per minute

soil gas - mixture of air and vapors in soil porosity

soil gas permeability - a soil's capacity to allow gas flow. The soil gas permeability varies according to grain size, soil uniformity, porosity, and moisture content.

soil matrix - soil as the environmental media containing contaminants

soil type - system of classification of soils based on physical properties

soil vapor extraction - an in situ soil aeration process designed and operated to maximize the volatilization of low-molecular-weight compounds, with somebiodegradation occurring

sorbed phase - the thin layer of material held near the surface of soil particles by physical and chemical interactions

sorption - general term for physical and chemical absorption and adsorption phenomena

substrate - the base on which an organism lives; reactant in microbial respiration reaction (electron donor)

surfactant - surface active agent that reduces the surface tension of liquids or the interfacial tension between two liquids or a liquid and a solid

TBC - to be considered

TPH - total petroleum hydrocarbons

treatability study - a planned group of measurements, bench-scale studies, or pilot-scale studies performed to measure the effectiveness of a process option forremediating a contaminated site or to collect design data for implementing a process option

TSCA - Toxic Substances Control Act

USACE - U.S. Army Corps of Engineers

USC - United States Code

USCS - Unified Soil Classification System

U.S. DoD - United States Department of Defense

U.S. EPA - United States Environmental Protection Agency

UST - underground storage tank

vacuum pump - a unit of mechanical equipment used to increase the pressure in a gas stream and providing a nearly complete vacuum at the suction flange

vapor pressure - the pressure exerted by a single component phase at a given temperature

vaporization - transfer of a chemical substance from the liquid or solid state to the gaseous state

VOA - volatile organic analysis

VOC - volatile organic compound

volatile - easily vaporized at relatively low temperatures

water table - planar surface between thevadose zone and the saturated zone

wetting - adding water to increase the moisture content of the soil prior to constructing the biopile or during biopile operation

### APPENDIX B

### OVERVIEW OF APPLICABLE FEDERAL ENVIRONMENTAL REGULATIONS

This appendix is divided into two sections. The first section covers major federal statutes and regulations related to site remediation and hazardous waste management. The second section gives an overview of various chemical-, location-, and action-specific regulations. This appendix is designed to give the reader an introduction to the various environmental regulations and statutes that may apply to the construction, operation, and loseout of a biopile facility. For more specific details, the reader should reference th*Code of Federal Regulations* or the *Federal Register*. Additionally, Appendices C and D list various state and U.S. EPA hazardous waste management points of contact.

**B.1** Summary of Major Federal Statutes and Regulations. This section provides general information about the major federal environmental regulations that may apply tobiopile remediation sites.

**B.1.1 Resource Conservation and Recovery Act.** The Resource Conservation and Recovery Act (RCRA) establishes rules for hazardous waste and underground storage tank (UST) corrective action plans. RCRA regulates wastes that are hazardous by characteristic (corrosive, ignitable, reactive, toxic) (40 CFR 261.20 through 261.24, "Subpart C, Characteristics of Hazardous Wastes") and specifically listed hazardous wastes (40 CFR 261.30 through 261.33, "Subpart D, Lists of Hazardous Wastes"). Federal UST regulations (40 CFR Part 280) do not provide specific soil cleanup standards, but methods for developing cleanup standards are included in the regulations.

**B.1.2** Clean Air Act of 1990 (42 USC 7401-7642). The Clean Air Act (CAA) established standards for vapor and particulate air emissions. In addition to these federal standards, local authorities usually have air release permitting requirements. These release standards vary widely and may range from little or no formal regulation to contaminant-specific mass discharge rates. Some authorities base their standards on the concentration at the nearest receptor, but others consider each site on a case-by-case basis.

**B.1.3** Clean Water Act (33 USC 1251-1376). The Clean Water Act (CWA) sets standards and requirements for pollutant discharge to surface waters. The National Pollutant Discharge Elimination System (NPDES) (40 CFR Parts 122 and 125) requires permits for the discharge of pollutants from any point source into the waters of the United States. General pretreatment regulations for publicly owned treatment works (POTW) are enforceable standards if remediation results in discharge to a POTW.

**B.1.4** Safe Drinking Water Act (40 USC 300). The Safe Drinking Water Act established both the National Primary Drinking Water Regulations (40 CFR Part 141) and National Secondary Drinking Water Regulations (40 CFR Part 143). Primary maximum contaminant levels (MCLs) are enforceable standards for contaminants in the public drinking water supply systems. These are set with regard to health factors, economic feasibility, and the technical feasibility of removing a contaminant from a water supply system. Secondary MCLs are intended as guidelines to protect the public welfare. These are contaminants which may adversely affect the aesthetic quality of drinking water such as taste, odor, color, and appearance and that may deter public acceptance of drinking water provided by public water systems.

Maximum contaminant level goals (MCLGs) exist for several organic and inorganic compounds found in drinking water. These arenonenforceable guidelines that consider only health factors.

**B.1.5** U.S. Water Quality Criteria, 1986. The water quality criteria (U.S. EPA, 1986) are concentrations of pollutants calculated for ambient surface water quality to protect human health and aquatic life. Criteria are set for both acute and chronic effects. These usually arenonenforceable concentrations unless they have been adopted by a state as part of the state's water quality standards. These criteria present useful guidance on the environmental effects of pollutants and may be included as conditions when setting remedial action objectives.

**B.2** Types of Regulations. This section outlines chemical-, location-, and action-specific regulations.

**B.2.1** Chemical-Specific Regulations. Chemical-specific regulations are numeric values, usually total limits or concentration ranges, for specific chemicals in water, soil, and air. These limits are either health-based or risk-based standards modified to consider the economic and technical possibility of implementation. Examples of this type of regulation include: soil, groundwater, and RCRA concentration-based treatment standards; MCLs for public drinking water; and National Ambient Air Quality Standards (NAAQS) for air quality. Table B-1 provides a review of some chemical-specific regulations.

Law/Regulation	Description
Resource Conservation and Recovery Act (RCRA); 40 CFR Parts 260-280	This act establishes MCLs for groundwater and treatment standards [based on best demonstrated available technologies (BDAT)] for hazardous wastes covered by the Land Disposal Restrictions (LDRs).
Federal Clean Air Act; 40 CFR Parts 50-80	This act sets limits on levels of air pollutants.
Federal Clean Water Act; 40 CFR Parts100-140, 400-470	This act sets limits on levels of water pollutants.
Safe Drinking Water Act; 40 CFR Parts141, 143, 260-280	This act and these regulations establishes MCLs and MCLGs for drinking water supplies.
Underground Injection Control Regulations; 40 CFR Parts 144-147	
U.S. Water Quality Criteria	These are criteria for ambient surface water quality established to protect human health and aquatic life. These are not standards and have no direct regulatory authority but may be included as to be considered (TBC) criteria.
Occupational Safety and Health Act (OSHA); 29 USC 651-678; 29 CFR Parts 1904, 1910, 1926	This act provides occupational safety and health requirements applicable to workers engaged in on-site field activities.

**B.2.2** Location-Specific Regulations. Location-specific regulations limit activity in an area, because of the unique or delicate nature of the site or its surroundings. These regulations vary widely with location. Some examples of locations which usually face constraints in remediation actions include protectedloodplains; wetlands; endangered species habitats; and archaeologically or historically important sites. Some federal location-specific regulations are reviewed by category in Table B-2.

Table B-2.	Review of 1	Potential I	Location-S	pecific	Regulations
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Location- Specific Factor	Citation	Requirement
Seismicity	Location Standards, Permitted Hazardous Waste Facilities; 40CFR 264.18	Facilities must not be located within 200feet of a fault that has been displaced in Holocene time.
Wilderness Areas, Wildlife	Fish and Wildlife Coordination Act; 16 USC 661 et seq.	This act requires that actions affecting fish and wildlife must include provisions to protect the affected fish and wildlife resources.
	Wild and Scenic Rivers Act; 16USC 1271 et seq.	This act protects designated rivers or river sections in the National Wild and Scenic Rivers System.
	Wilderness; Act 16 USC 1131 etseq.	This act protects and restricts activities in areas designated as part of the National Wilderness Preservation System.

Location- Specific Factor	Citation	Requirement
Wetlands, Floodplains	Executive Order 11990; Protection of Wetlands; 40 CFR Part 6, Appendix A; 40 CFR 6.302(a)	Federal agencies must take action to avoid adverse impacts, to minimize potential harm, and to preserve and enhance wetlands to the extent possible.
	Executive Order 11988; Protection of Floodplains	Adverse effects associated with the development of a floodplain must be evaluated.
	40 CFR Part 6, Appendix A	Federal agencies are required to incorporate floodplain management goals and wetlands protection considerations in their planning, regulatory, and decision-making process.
	40 CFR Part 6, Appendix A	Federal agencies should avoid new construction in wetlands areas.
	Clean Water Act §404; 40 CFR 230.10; 33 CFR Parts 320-330	This act prohibits discharge of dredge or fill material into wetlands without a permit.
Wetlands, Floodplains (continued)	Clean Water Act §404(b)(1)	This act provides for the enhancement, restoration, or creation of alternative wetlands.

	Location Standards, Permitted Hazardous Waste Facilities; 40CFR 264.18	This regulation requires facility design to prevent washout of hazardous waste.	
Critical habitat upon which an endangered or threatened species depends	Endangered Species Act of 1973; 16 USC 1531 et seq.; 50 CFR Parts 200 and 402	Individuals must take action to conserve endangered or threatened species and must not destroy or adversely modify critical habitat.	
	Fish and Wildlife Coordination Act; 16 USC 661 et seq.	Individuals must consult with Department of Interior, Fish and Wildlife Service, and state personnel required to ascertain that proposed	
	Fish & Wildlife Conservation Act 16 USC 2901	actions will not affect any listed species.	
Within areas where action may cause irreparable harm, loss, or destruction of significant artifacts	Archaeological and Historic Preservation Act; 16 USC 469 to 469c-1	Individuals must take action to recover and preserve artifacts.	
Property included in or eligible for the National Registry of Historic Places	National Historic Preservation Act; 16 USC 470 et seq.; 36 CFR 800.1	Individuals must take action to preserve historic properties and plan actions to minimize harm to National Historic Landmarks.	
Registry of Flistone Flaces	National Historic Landmarks Program; 36 CFR 65		
	National Register of Historic Places; 36 CFR Part 60		
	Executive Order 11593; 36 CFR 800.4	Federal agencies must identify possible effects of proposed remedial activities on historic properties, and measures must be implemented to minimize or mitigate potential effects.	
Archaeological sites or resources on public land	Archaeological Resources Protection Act of 1979; 16 USC 470aa-11; 43 CFR Part 7	Individuals must take steps to protect resources and to preserve data.	

**B.2.3** Action-Specific Regulations. Action-specific regulations, summarized in Table B-3, are rules that apply to specific technologies or activities. Examples of some action-specific regulations include monitoring requirements; effluent and leachate discharge limits from specific processes; hazardous waste manifesting requirements; and worker health and safety requirements. Air and water discharges from theoiopile will be required to meet permitting standards set by local jurisdictions.

Table B-3. R	eview of Potential	Action-Specific	Regulations
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Law/Regulation	Comments
Resource Conservation and Recovery Act (RCRA) EPA Regulations for Hazardous Waste; 40 CFR Parts 260 through 270	These rules establish a comprehensive cradle-to-grave program for safe management of hazardous waste. Contaminated soils that are sufficiently similar to hazardous waste under RCRA or state regulations may be affected by these regulations as applicable or relevant and appropriate requirements (ARARs) or as to be considered (TBC) guidance.
EPA Underground Storage Tank Requirements; 40 CFR Part 280	These regulations provide a regulatory program for underground storage tanks.
Toxic Substances Control Act; 15 USC 2601 et seq.	This act provides for regulation of specific toxic substances, including polychlorinated biphenyls (PCBs).
Federal Insecticide, Fungicide, andRodenticide Act; 7 USC 136 et seq.	This act provides for regulation of the production, use, and disposal of pesticides.
National Environmental Policy Act; 42 USC 4321 et seq.	This act requires that all major federal actions be evaluated for potential impacts on the environment.
Pollution Prevention Act of 1990; 42 USC 13101 et seq.	This act encourages eliminating or reducing the production of hazardous or toxic wastes at the source and responsible recycling.

**B.3 Regulatory Summary.** Not all of the previous regulations apply specifically tobiopiles, although they do apply to general environmental remediation. Some regulations do apply to certain parts of theorem process. The CAA regulates vapor emissions frombiopiles, and the CWA regulates fluid emissions frombiopiles. Disposal of some soils containing wastes regulated by the Toxic Substances Control Act (TSCA) or by RCRA may be prohibited in certain landfills or in certain mixtures. Occupational Safety and Health Act (OSHA) standards must be followed to protect field personnel. Location-specific regulations also may apply specifically tobiopiles.

#### B.4 Reference for Appendix B

U.S. Environmental Protection Agency. 1986. *Quality Criteria for Water 1986*. EPA 440/5-86-001. Office of Water, Washington, DC. May 1. (Also with Update No. 1, 1986, and Update No. 2, 1987.)

### APPENDIX C

#### STATE ENVIRONMENTAL REGULATORY AGENCIES

#### ALABAMA

Alabama Dept of Environmental Management Land Division 1751 Federal Drive Montgomery, AL 36130 334-271-7730

#### ALASKA

Dept. of Environmental Conservation 410 Willoughby Avenue, Suite 105 Juneau, AK 99801-1795 Program Manager: 907-465-5150

Northern Regional Office 610 University Avenue Fairbanks, AK 99709

#### ARIZONA

Arizona Dept. of Environmental Quality Waste Programs Bureau 3033 North Central Avenue Phoenix, AZ 85012 602-207-2300

#### ARKANSAS

Dept. of Pollution Control and Ecology Hazardous Waste Division P.O. Box 8913 8001 National Drive Little Rock, AR 72209-8913 501-682-0833

#### CALIFORNIA

California EPA Dept. of Toxic Substances Control 400 P Street, 4th Floor P.0. Box 806 Sacramento, CA 95812-0806 916-322-0504

California EPA State Water Resources Control Board Water Resources Control Board P.O. Box 100 Sacramento, CA 95812-0100 916-657-2390

#### COLORADO

Public and Environment Dept. Hazardous Materials and Waste Management Division 4300 Cherry Creek Drive South Denver, CO 80222 303-692-3300

#### CONNECTICUT

Dept. of Environmental Protection Waste Management Bureau Waste Engineering and Enforcement Division 79 Elm Street Hartford, CT 06106 203-424-3023

Connecticut Resource Recovery Authority 179 Allyn Street, Suite 603 Professional Building Hartford, CT 06103 203-549-6390

#### DELAWARE

Dept. of Natural Resources and Environmental Control Division of Air and Waste Management Hazardous Waste Office 89 King's Highway P.O. Box 1041 Dover, DE 19903 302-739-3689

#### DISTRICT OF COLUMBIA

Dept. of Consumer and Regulatory Affairs Environmental Regulation Administration Pesticides and Hazardous Waste Management Branch 2100 Martin Luther King Avenue, SE, Suite 203 Washington, DC 20020 202-645-6617

#### FLORIDA

Environmental Protection Dept. Waste Management Division Solid and Hazardous Waste Bureau 2600 Blair Stone Road Tallahassee, FL 32399-2400 904-488-0300

#### GEORGIA

Georgia Dept. of Natural Resources Environmental Protection Division Hazardous Waste Management Branch Floyd Towers East, Suite 1154 205 Butler Street, SE Atlanta, GA 30334 404-656-7802

#### HAWAII

Dept. of Health Solid and Hazardous Waste Branch 919 Alamoana Boulevard, Room 212 Honolulu, HI 96814 808-586-4225

#### IDAHO

Dept of Health and Welfare Division of Environmental Quality 1410 N Hilton Boise, ID 83706 208-334-5840

#### ILLINOIS

Department of Commerce and Community Affairs Solid Waste Division 325 West Adams Street Springfield, IL 62704 217-785-2800

#### INDIANA

Dept. of Environmental Management Office of Solid and Hazardous Waste 100 North Senate Avenue Indianapolis, IN 46204 317-232-3210

#### IOWA

Dept. of Natural Resources Waste Management Assistance Division 502 E 9th Street Wallace State Office Building Des Moines, IA 50319-0034 515-281-8681

#### KANSAS

Dept. of Health and Environment Bureau of Waste Management Forbes Field, Building 740 Topeka, KS 66620 913-296-1612

#### KENTUCKY

Natural Resources and Environmental Protection Cabinet Division of Waste Management 14 Reilly Road Frankfort, KY 40601 502-564-6716

#### LOUISIANA

Dept. of Environmental Quality Solid and Hazardous Waste Division 7290 Bluebonnet Boulevard Baton Rouge, LA 70810 504-765-0249

#### MAINE

Dept. of Environmental Protection Bureau of Hazardous Materials and Solid Waste Control Hospital Street Ray Building State House Station #17 Augusta, ME 04333 207-287-2651

#### MARYLAND

Environment Dept. Waste Management Administration 2500 Broening Highway Baltimore, MD 21201 410-631-3304

#### MASSACHUSETTS

Dept. of Environmental Protection Hazardous Waste Division One Winter Street, 7th Floor Boston, MA 02108 617-292-5853

#### MICHIGAN

Michigan Dept. of Natural Resources Waste Management Division 608 West Allegen Lansing, MI 48933 517-373-2730

#### **MINNESOTA**

Pollution Control Agency Hazardous Waste Division 520 Lafayette Road North St. Paul, MN 55155-4194 612-297-8502

#### MISSISSIPPI

Dept. of Environmental Quality Division of Solid and Hazardous Waste Management 2380 Highway 80 West Jackson, MS 39204 601-961-5047

#### MISSOURI

Dept. of Natural Resources Waste Management Program 205 Jefferson, 13th Floor Jefferson City, MO 65101 314-751-3176 Missouri Natural Resources Hotline: 800-334-6946

#### MONTANA

Dept. of Health and Environmental Sciences Waste Management Division P.O. Box 200901 Helena, MT 59620-0901 406-444-1430

#### NEBRASKA

Environmental Quality Dept. P.O. Box 98922 1200 N Street, Suite 400 Lincoln, NE 68509 402-471-2186

#### NEVADA

Conservation and Natural Resources Dept. Division of Environmental Protection Waste Management Program 123 West Nye Carson City, NV 89710 702-687-4670

#### **NEW HAMPSHIRE**

Dept. of Environmental Services Waste Management Division Health and Welfare Building 6 Hazen Drive Concord, NH 03301 603-271-2900

#### NEW JERSEY

Dept. of Environmental Protection and Energy Solid Waste Management 120 S Stockton Street, CN 414 Trenton, NJ 08625-0414 609-984-6880

#### **NEW MEXICO**

Environmental Improvement Division Hazardous Waste Bureau P.O. Box 26110 1190 St. Francis Santa Fe, NM 87502 505-827-2775

#### NEW YORK

Dept. of Environmental Conservation Division of Solid Waste and Hazardous Materials 50 Wolf Road, Room 488 Albany, NY 12233 518-457-6934 SQG Hotline: 800-462-6553

#### NORTH CAROLINA

Dept. of Environmental, Health, and Natural Resources Hazardous Waste Section 512 North Salisbury Street Raleigh, NC 27604 919-715-4140

#### NORTH DAKOTA

Dept. of Health Consolidated Laboratories Division of Waste Management 1200 Missouri Avenue Bismarck, ND 58504 701-328-5166

#### OHIO

Ohio EPA Division of Hazardous Waste 1800 Watermark Drive Columbus, OH 43215-1099 614-644-2917

#### OKLAHOMA

Environmental Quality Dept. Hazardous Waste Management Division 1000 NE Tenth Street Oklahoma City, OK 73117 405-271-5338

#### OREGON

Dept. of Environmental Quality Hazardous Waste Division 811 SW Sixth Avenue Portland, OR 97204 503-229-5913

#### PENNSYLVANIA

Dept. of Environmental Resources Bureau of Waste Management Director's Office 400 Market Street P.O. Box 2063 Harrisburg, PA 17105 717-787-9870

#### RHODE ISLAND

Dept. of Environmental Management Division of Waste Management 291 Promenade Street Providence, RI 02908 401-277-4700

#### SOUTH CAROLINA

Dept. of Health and Environmental Control Bureau of Solid and Hazardous Waste Management 2600 Bull Street Columbia, SC 29201 803-734-5202

#### SOUTH DAKOTA

Dept. of Environment and Natural Resources Office of Waste Management 523 East Capital Avenue Pierre, SD 57501 605-773-3351

#### TENNESSEE

Environment and Conservation Dept. Solid Waste Management Division 401 Church Street, 5th Floor Nashville, TN 37243-1535 615-532-0780

#### TEXAS

Natural Resource Conservation Commission Industrial and Hazardous Waste Division P.O. Box 13087 Capital Station Austin, TX 78711-3087 512-239-2334

#### UTAH

Dept. of Environmental Quality Division of Solid and Hazardous Waste P.O. Box 144880 Salt Lake City, UT 84114-4880 801-538-6170

#### VERMONT

Natural Resources Agency Environmental Conservation Dept. Waste Management West Office Building 103 South Main Street Waterbury, VT 05671-0404 802-241-3888

#### VIRGINIA

Natural Resources Office Environment Quality Dept. 629 East Main Street Richmond, VA 23219 804-698-4020 Hazardous Waste Hotline: 800-552-2075

#### WASHINGTON

Dept. of Ecology Solid and Hazardous Waste Program P.O. Box 47600 Olympia, WA 98504-7600 360-407-6103

#### WEST VIRGINIA

Environmental Protection Bureau Waste Management Division 1356 Hansford Street Charleston, WV 25301 304-558-5929

#### WISCONSIN

Dept. of Natural Resources Solid and Hazardous Waste Management P.O. Box 7921 101 S Webster Street Madison, WI 53707 608-266-1327

#### WYOMING

Dept. of Environmental Quality Solid Waste Management Division Herschler Building 122 West 25th Street Cheyenne, WY 82002 307-777-7752

#### **EPA Region 1**

JFK Federal Building 1 Congress Street Boston, MA 02203 (617) 565-3420

Connecticut, Massachusetts, Maine, New Hampshire, Rhode Island, Vermont

#### **EPA Region 2**

290 Broadway New York, NY 10007-1866 (212) 637-5000

New Jersey, New York, Puerto Rico, Virgin Islands

#### **EPA Region 3**

841 Chestnut Street Philadelphia, PA 19107 (215) 597-9800

Delaware, District of Columbia, Maryland, Pennsylvania, Virginia, West Virginia

# EPA Information on the World Wide Web:

http://www.epa.gov

### APPENDIX D

#### **EPA REGIONAL OFFICES**

#### **EPA Region 4**

345 Courtland Street NE Atlanta, GA 30365 (404) 347-4727

Alabama, Florida, Georgia, Kentucky, Mississippi, North Carolina, South Carolina, Tennessee

#### EPA Region 5

77 West Jackson Blvd. Chicago, IL 60604 (312) 353-2000

Illinois, Indiana, Michigan, Minnesota, Ohio, Wisconsin

#### EPA Region 6

1445 Ross Avenue Dallas, TX 75202-2733 (214) 655-6548

Arkansas, Louisiana, New Mexico, Oklahoma, Texas

### **EPA Region 7**

726 Minnesota Avenue Kansas City, KS 66101 (913) 551-7000

Iowa, Kansas, Missouri, Nebraska

#### EPA Region 8

One Denver Place 999 18th Street Denver, CO 80202-2406 (303) 312-6312

Colorado, Montana, North Dakota, South Dakota, Utah, Wyoming

#### EPA Region 9

75 Hawthorne Street San Francisco, CA 94105 (415) 744-1305

Arizona, California, Hawaii, Nevada, American Samoa, Guam, Trust Territories of the Pacific

#### EPA Region 10

1200 Sixth Avenue Seattle, WA 98101 (206) 553-1200

Alaska, Idaho, Oregon, Washington

## ADDITIONAL SOURCES OF INFORMATION

### Phone & Hotline Information

- RCRA/Superfund Hotline
   1-800-424-9346 (in Washington, DC 260-3000)
- Safe Drinking Water ActHotline 1-800-426-4791
- EPA Small Business OmbudsmarHotline
   1-800-368-5888 (in Washington, DC 557-1938)
- National Response Center
   1-800-494-8802 (in Washington, DC 260-2675)
- Transportation of Hazardous Materials 202-366-4488
- Toxic Substances Control Act (TSCA) Assistance Serine 202-554-1404
- Center for Hazardous Materials Research (CHMR)Hotline 1-800-334-2467

#### APPENDIX E

#### CHEMICAL AND PHYSICAL PROPERTIES OF PETROLEUM HYDROCARBONS

Fuels and waste oils will partition by volatilization, dissolution, or adsorption of individual components depending on their physical and chemical properties. The fate and transport of fuels and waste oils duringiopile treatment depend mainly on their vapor pressure, water solubility, and Henry's law constant (U.S. Department of Health & Human Services, 1993a,b,c). Physical and chemical data for some types of fuel and oil components are illustrated in Figure E-1 and listed in Table E-1.

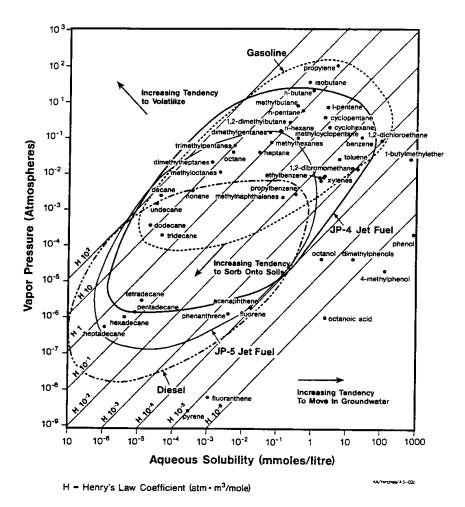


Figure E-1. Illustration of the Effect of the Properties of Fuel Components on In Situ Transport.

emical			Molecular	Density	Melting Point	<b>Boiling Point</b>	Vapor Pressure	Solublitty in Water
Class	Compound	Formula	Weight	mg/L (at °C)	() ()	() -	mm Hg (at °C)	mg/L (at *C)
2	n-Butane	CH,CH,CH,CH,	58.12	0.600 (0)	-135	-0.6	760 (-0.5)	61.4 (25)
	n-Pentane	CH3(CH3),CH3	72.15	0.630 (18)	-129.7	36.3	400 (18.5)	40 (25)
	2-Methylpentane	сн,(сн,),сн(сн,),	86.18	0.653(*)	-154	62	200 (24.1)	14 (25)
	n-Hexane	CH3(CH3, CH3	86.17	0.659 (20)	-94	69	100 (15.8)	10 (25)
	2-Methylhexane	CH3(CH3)3CH(CH3)2	100.21	0.679 <sup>(a)</sup>	-118	06	60 (23)	2 (25)
	3-Methylhexane	CH,(CH,),CH(CH,)CH,CH	100.21	0.687 <sup>(a)</sup>	-119	91	60 (24.5)	4 (25)
	n-Heptane	CH,(CH),CH,	100.20	0.684 (20)	9.06-	98.4	40 (22.3)	3 (25)
	2-Methylheptane	CH3(CH3),CH(CH3),	114.23	0.698 <sup>(a)</sup>	No data	116	20 (24.4)	No data
	3-Methylheptane	CH3(CH3)3CH(CH3)CH2CH3	114.23	No data	- 120.5	115	20 (25.4)	0.8 (25)
	n-Octane	сн,(сн,),сн,	114.22	0.703 (20)	-58.5	125.7	10 (19.2)	0.6 (25)
	n-Undecane	CH,(CH),CH	156.30	0.741 (20)	-25.6	194.5	1 (32.7)	0.04 (25)
	n-Dodecane	сн,(сн,),осн,	170.33	0.751 (20)	9.6-	214.5	1 (47.8)	0.004 (25)
	n-Tetradecane	CH3(CH3),2CH3	198.38	0.765 (20)	5.5	252.5	1 (76.4)	0.002 (25)
alkanes	Methylcyclopentane	C,H,CH,	84.16	0.749 <sup>(a)</sup>	-142	72	100 (17.9)	42 (25)
	Cyclohexane	cH <sub>3</sub> (cH <sub>3</sub> cH <sub>3</sub> ),cH <sub>3</sub>	84.16	0.779 (20)	6.5	80 - 81	100 (25.5)	60 (25)
	Methylcyclohexane	CH <sub>2</sub> (CH <sub>2</sub> CH <sub>2</sub> ),CHCH	98.18	0.769 (20)	-126.3	101	40 (22)	14 (25)
	n-Butytcyclohexane	C.H.,(CH_),CH,	140.27	0.818 <sup>(a)</sup>	-78	178 - 180	No data	No data
enzenes	Benzene	°H°	78.11	0.880 (20)	5.5	80.1	76 (20)	1,800 (20)
	Toluene	C,H,CH,	92.10	0.870 (20)	-95.1	110.8	22 (20)	640 (25)
	Ethylbenzene	C <sub>6</sub> H <sub>5</sub> C <sub>2</sub> H <sub>5</sub>	106.17	0.867 (20)	-94,4	136.2	10 (25.9)	100 (15)
	o-Xylene	C,H,(CH3)2	106.17	0.881 (20)	-25	144.4	5 (20)	187 (25)
	m-Xylene	C,H,(CH,),	108.17	0.861 (20)	-47.3	139.3	6 (20)	166 (25)
	p-Xylene	C,H,(CH,),	108.17	0.864 (20)	13	138.4	6.5 (20)	175 (25)
	1,2,4-Trimethylbenzene	c,HJ,(CHJ),	120.19	0.876 (20)	- 44	169 - 171	1 (13.6)	(q) <sup>j</sup>
	1,3,5-Trimethylbenzene	C <sub>e</sub> H <sub>3</sub> (CH <sub>3</sub> ),	120.19	0.865 (20)	-45, -52	164,8	1 (9.6)	(a)
thalenes	Naphthalene	C. <sub>0</sub> H	128.16	1.145 (20)	80.2	217.9	0.0082 (25)	30 (25)
	1-Methylnaphthalene	C <sub>10</sub> H,CH,	142.19	1.025 (14)	-19	244.6	No data	(q) <sup>!</sup>

Table E-1. Properties of Example Components of Fuels and Waste Oils

### **References for Appendix E**

Dean, J.A. 1992. Lange's Handbook of Chemistry, 14th ed. McGraw-Hill, New York, NY.

Lide, D.R., and H.P.R. Frederikse (Eds.). 1993. CRC Handbook of Chemistry and Physics, 74th ed. CRC Press, Boca Raton, FL.

Mackay, D., and W.Y. Shiu. 1981. "A Critical Review of Henry's Law Constants for Chemicals of Environmental Interest." *J. Phys. Chem. Ref. Data*, *10* (4):1175.

Verschueren, K. 1983. *Handbook of Environmental Data on Organic Chemicals*, 2nd ed. Van Nostrand Reinhold Co., New York, NY.

#### APPENDIX F

### COMPANIES DOING TREATABILITY STUDIES

A listing of examples of some companies providing treatability testing fobiopile treatment is provided to assist the user in starting a search for an appropriate testing service. This listing is not an exhaustive review of all possible qualified services. **Inclusion on the list should in no way be interpreted as an endorsement of any of the companies.** 

Battelle Environmental Restoration Department 505 King Avenue Columbus, Ohio 43201 (614) 424-4698

Biotrol, Inc. 10300 Valley View Road Suite 107 Eden Prairie, Minnesota 55344 (612) 942-8032

Center for Environmental Microbiology 1660 Chicago Avenue Suite M-2 Riverside, CA 92507 (909) 787-3405

Lawhon & Associates 6300 Proprietor's Road Worthington, Ohio 43085 (614) 436-8400

Microbe Inotech Laboratories, Inc. 12133 Bridgestone Square Drive Saint Louis, MO 63044 (314) 344-3030

Organic Waste Systems 3155 Research Boulevard Suite 104 Dayton, Ohio 45420 (513) 253-6888 Retec 9 Pond Lane Concord, Massachusetts 01742 (508) 371-1422

Retec 1005 W 9th Avenue Suite A King of Prussia, Pennsylvania 19406 (610) 992-9950

Roy F. Weston 212 Frank West Circle Stockton, California 95206 (209) 983-1340

Versar, Inc. 39830 Grant River Novi, Michigan 48375 (703) 893-4106

Woods End Laboratory Old Rome Road Route 2 Box 1850 Mt. Vernon, Maine 04352 (207) 293-2453

#### APPENDIX G

#### METHODS OF DETERMINING FIELD CAPACITY

Field capacity (FC) is "...the amount of water held in the soil after the excess gravitational water has drained away [from a saturated soil] and after the rate of downward movement of water has materially decreased" (Klute, 1994). This definition assumes that (a) the soil is deep and permeable; (b) no evaporation occurs from the soil surface; and (c) no barriers to permeability occur at shallow depths in the soil. In other words it is the water left in the soil after extra water from rainfall or irrigation has drained from the area or drainage has slowed to a negligible rate. The field capacity of a soil can be determined by ASTM D2325 or ASTM 3152, or may be approximated by any of the methods described in G.1 through G.6.

**G.1 In Situ Field Capacity.** When soil is at its naturally driest condition, construct a watertight dike around the perimeter of the test zone, which could be a  $10 \text{ f} \times 10$  ft area. Flood the soil surface of the test area with water if it is flat, but if the area slopes then use a sprinkler system to apply the water. Immediately following water application, cover the test area with an evaporation barrier. After 48 hours, take soil samples, at least 4 at each depth.

$$FC_{w} = M_{w}/M_{s}$$
(1)

where:  $M_w$  = the mass of water added  $M_s$  = the oven-dried mass of soil  $FC_w$  = the field capacity by weight

or  $FC_v = FC_w X r_b / r_w = M_w / V_a r_w$ (2)

$$\label{eq:Va} \begin{split} where: FC_v &= the \ field \ capacity \ by \ volume \\ V_a &= the \ bulk \ soil \ volume \\ r_b &= the \ bulk \ soil \ density \\ r_w &= the \ density \ of \ water \end{split}$$

This method is the best for determining FC.

**G.2 Large Soil Core Method (Approximate).** Obtain a soil core with a 16-gauge steel cylinder. Remove it carefully from the ground, trying to maintain natural sorting and packing. In a laboratory, place the cylinder, with bottom cover removed, on a more fine-grained, dry soil. Add water to the core surface until it infiltrates to the desired depth then let stand for 48 hours. Take samples and calculate the approximate FC using Equations 1 and 2.

**G.3 Small Soil Core Method (Approximate).** Take several soil cores from each soil depth at the test site. Maintain the natural structure. In the laboratory, trim the ends of the soil core until they are even with the retaining ring. Place the core and ring on the ceramic plate, rotating slightly to ensure good contact. Wet the soil by capillary rise over 12 hours. Slowly increase the water level until the entire core is submerged and saturated. Then remove the excess water using a pressure plate with pressure equal to 10kPa. Continue draining water until no more flows from the pressure plate. Oven-dry the sample for 24 hours at 10<sup>s</sup>C. Calculate the water content on either a weight or volume basis.

**G.4 Centrifuge Method (Approximate).** Perform this determination in duplicate. Cover the screened area of the brass centrifuge cups withWhatman no. 2 filter paper. Place 30 g of air-dried soil that has been screened through a 2-mm sieve into each cup. Wet the samples from the bottom by placing them in water and letting them soak for a few hours. Load the wet samples into the centrifuge and as quickly as possible bring them to 2,468 revolutions/minute. Maintain this speed for 30 minutes. After centrifugation is finished, quickly transfer the soil to tared moisture cans and obtain the masses needed to calculate FC.

**G.5** Field-Expedient Method (Approximate). Figure G-1 shows the steps to be followed to estimate field

capacity by the field-expedient method.

#### FIELD-EXPEDIENT METHOD FOR ESTIMATING THE FIELD CAPACITY OF A SOIL

- 1. Cut the bottom off a 2-L bottle, cover and tape some sort of screening material to the top opening, and invertite bottle.
- 2. Fill the bottle half full with a sample of dry soil to be used in thebiopile.
- 3. Weigh bottle with soil and record weight.
- 4. Add water to soil in bottle. Mix while adding and allow water to saturate soil. Be careful not to spill any of the soil slurry from bottle.
- 5. Cover the bottle to prevent evaporative losses.
- 6. Let bottle stand overnight with the screened top down to allow water to drain from soil.
- 7. Weigh bottle with wet soil and record weight.
- Estimate amount of water required to reach field capacity: [(wet soil - dry soil) / dry soil] =lb water required to reach field capacity perlb soil. To convert lb of water to gal of water multiplylb of water by 8.36.

Figure G-1. Field-Expedient Method for Estimating the Field Capacity of a Soil.

## G.6 Reference for Appendix G

Klute, A. (Ed.). 1994. Methods of Soil Analysis, Part 1. 2nd ed. American Society of Agronomy, Madison, WI.

## APPENDIX H

## **BIOPILE COST ESTIMATOR® COST SHEETS FOR SAMPLE CASES**

## H.1 TEMPORARY BIOPILE SAMPLE COST ESTIMATES

Total Temporary Biopile Installation Costs:

•\_\_

Project Name: Sample Test

Site Location: Test Site Date of Estimate: 6/9/96 Total Volume to be Treated: 500 yd<sup>3</sup>

Unit Cell Dimensions: 50 ft x 60 ft

 Number of Cells:
 i
 Cell(s)

 Volume of Cell Size:
 500
 yd³/cell

 Projected Cycle Length:
 4
 months

 Total Project Life:
 4
 months

yd<sup>3</sup> OVA Rental Cell(s) OVA Parchase yd<sup>3</sup>/cell Nene months Injection months Extraction

\$29,973

Item	Unit	Unit Cost	Quantity	Total Cost	Comments	Vendor
treated 4-inch x 4-inch x 8' lumber	each	\$5.27	28	\$147.56		Fifth Avenue Lumber
treated 2-inch x 4-inch x 8' lumber	each	\$4.42	28	\$123.76		Fifth Avenue Lumber
3/4-in x 6-in lag bolt w/ washer and nut to secure 4-in x 4-in beams	each	\$1.72	56	\$96.32		Columbus Fasteners
connecting brackets to join 4-inch x 4-inch beams	each	\$0.52	56	\$29.12		Columbus Hardware
60-mil 51-ft x 61-ft HDPE bottom liner	each	\$1,500.00	1	\$1,500.00		Racer
clean dirt or clay for 8-inch foundation	yd <sup>3</sup>	\$10.00	7.1	\$71.00		
4-inch flexible, slotted drainage	n fi	\$0.18	90	\$16.20		Jones Fuel Company
	+					Discount Drainage
4-inch end caps for drainage pipe	each	\$10.22	3	\$30.66		U.S. Plastics
2-inch schedule 40 PVC pipe	20 ft	\$23.00	3	\$69.00		U.S. Plastics
4-inch rubber unions to connect drainage pipe to PVC	each	\$5.89	3	\$17.67		Discount Drainage
2-inch PVC gate valves	each	\$20.00	3	\$60.00		Pipe-Valves Inc.
4-inch SCH 40 PVC threaded/slip coupling	each	\$7.48	6	\$44.88		U.S. Plastics
4-inch to 2-inch PVC reducing bushing	each	\$25.15	3	\$75.45		U.S Plastics
2-inch PVC slip tee	each	\$2.31	2	\$4.62		U.S. Plastics
2-inch PVC slip elbow (will vary depending on site layout)	each	\$2.51	2	\$5.02	· · · · · · · · · · · · · · · · · · ·	U.S. Plastics
2-inch SCH 40 PVC pipe (adjust as dictated by proximity of pump)	20 ft	\$23.48	8	\$187.81		U.S. Plastics
20-gal heavy duty PVC water knockout tank	each	\$151.75	1	\$151.75		U.S. Plastics
point switch/alarm controller (level control)	each	\$300.00	1	\$300.00		Cole-Parmer
fiber-optic sensor (for level controller)	each	\$215.00	1	\$215.00		Cole-Parmer
1-inch SCH 40 PVC pipe (adjust as required to plumb)	20 ft	\$10.99		\$215.00		
						U.S. Plastics
500-gal water collection tank	each	\$860.00	1	\$860.00		Cole-Parmer
1-1/2 hp rotary positive displacement vacuum pump	each	\$821.00	1	\$821.00		Grainger
4" thick concrete pad (10' x 12') for storage shed	sq ft	\$6.00	120	\$720.00		General Maintenance & Eng. co
storage shed to house pump and materials	each	\$249.00	1	\$249.00		Grainger
55-gal granular activated carbon drum	each	\$496.00	2	\$992.00		Carbtrol Corp
misc. PVC fittings to connect knockout tank, pump and carbon drums		\$250.00	1	\$250.00		estimate
20-mil 70-ft x 75-ft plastic sheeting (HDPE or qualified substitute)		\$200.00	1	\$200.00		estimate
1-inch x 3-inch x 8-ft furring grade wood	each	\$2.56	28	\$71.68		Fifth Avenue Lumber
5/16-inch x 3-inch hex head sheet metal screw	each	\$0.10	150	\$15.00		Dotter fastener
nylon rope, 5/16"	100 ft	\$19.23	4	\$76.92		Grainger
3/8-inch x 3-1/4-inch   bolt	each	\$0.76	12	\$9.12		Columbus Fasteners
Urea	50 lb bag	\$9.00	58	\$522.00		Lesco
Ammonium Phosphate, Dibasic	50 lb bag	\$44.50	- 8	\$356.00		Ashland Chemical
Potassium Sulfate	50 lb bag	\$16.20	<u> </u>	\$0.00		
Superphosphate	50 lb bag	\$5.75				Lesco
water hoses with nozzles	-			\$0.00		Lesco
scoop for nutrient addition	each	\$33.69	4	\$134.76		Columbus Hardware
		\$5.00	1	\$5.00		K-Mart
scale to measure nutrients	each	\$68.05	1	\$68.05		Grainger
2 1/2-gal bucket for holding/distributing nutrients	each	\$5.45	1	\$5.45		Cole-Parmer
1/4-inch OD nylon tubing for monitoring points-green	50ft	\$19.25	2	\$38.50		Cole-Parmer
1/4-inch OD nylon tubing for monitoring points-red	50ft	\$19.25	3	\$57.75		Çole-Parmer
1/4-inch OD nylon tubing for monitoring points-blue	50ft	\$19.25	3	\$57.75		Cole-Parmer
Thermocouple display, hand held	each	\$75.00	1	\$75.00		
K-type thermocouple wire, 24-gauge	100 ft roll	\$48.50	1	\$48.50		Cole-Parmer
K-type thermocouple plug	each	\$3.18	10	\$31.80		LH Marshall
NPT 3/8 X 1/4 tube fitting	each	\$1.31	10	\$13.10		Forberg Scientific
female quick-connect	each	\$12.10	13	\$157.30		Forberg Scientific
gravel for strainer	50 lb	\$20.00	1	\$20.00		estimate aquanum supply
suction strainer 3/4"	each	\$9.49	10	\$94.90		Grainger
3/8" thread tap	each	\$6.32	10	\$63.20		Columbus Hardware
male guick connect X 1/4 " tubing	each	\$11.88	13	\$154.44		
quick-connect protectors	each	\$5.01	13	\$65.13		Forberg Scientific
shovel		\$20 95				Forberg Scientific
1/3 HP sump pump	each		2	\$41.90		Grainger
	each	\$131.78		\$131.78		U.S. Plastics
1/3 HP transfer pump	each	\$240.50	_1	\$240.50		Grainger
Gas Tech 3250X 02/C02	each	\$3,700.00	1	\$3,700.00		Control Analytics
Gas Tech GT105 O2/TPH	each	\$1,548 75	1	\$1,548.75		Control Analytics
AVO		\$0.00	1	\$0.00	None	Hazco
Soil Compaction: technician					Labor	
	hour	\$55.00	16	\$880.00		
Berm and liner installation: technician	hour hour		16 48	\$880.00 \$2,640.00	Labor	
Berm and liner installation: technician Berm and liner instalation supervision: supervisor		\$55.00				
Berm and liner instalation supervision: supervisor	hour hour	\$55.00 \$55.00 \$80.00	48 24	\$2,640.00 \$1,920.00	Labor Labor	
Berm and liner instalation supervision supervisor Knockout tank installation, technician	hour hour hour	\$55.00 \$55.00 \$80.00 \$55.00	48 24 16	\$2,640.00 \$1,920.00 \$880.00	Labor Labor Labor	
Berm and liner instalation supervision supervisor Knockout tank installation, technician Transpt/insti leachate collection tank, laborer	hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00	48 24 16 16	\$2,640,00 \$1,920,00 \$880,00 \$320,00	Labor Labor Labor Labor Labor	
Berm and liner instalation supervision supervisor Knockout tank installation, technician Transpt/instl leachate collection tank, laborer Install blower w/ vanable speed process control, electrician	hour hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00 \$75 00	48 24 16 16 6	\$2,640.00 \$1,920.00 \$880.00 \$320.00 \$450.00	Labor Labor Labor Labor Labor Labor	
Berm and liner instalation supervision supervision Knockout tank installation, technician Transpt/insti leachate collection tank, laborer install blower w/ vanable speed process control electrician install knockout tank liquid level controller electrician	hour hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00 \$75 00 \$75 00	48 24 16 16 6 6	\$2,640,00 \$1,920,00 \$880,00 \$320,00 \$450,00 \$450,00	Labor Labor Labor Labor Labor Labor	
Berm and liner installation supervision supervision Knockout tank installation, technician Transplinist leachate collection tank, laborer install blower w/ vanable speed process control, electrician install knockout tank liquid level controller, electrician soil pile construction, loader operator	hour hour hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00 \$75 00 \$75 00 \$75 00 \$55 00	48 24 16 16 6 6 16	\$2,640.00 \$1,920.00 \$880.00 \$320.00 \$450.00 \$450.00 \$880.00	Labor Labor Labor Labor Labor Labor Labor	
Berm and liner installation supervision supervision Knockout tank installation technician Transpt/inst leachate collection tank laborer install blower w/ vanable speed process control electrician install knockout tank liquid level controller electrician soil pile construction loader operator soil pile construction technician	hour hour hour hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00 \$75 00 \$75 00 \$55 00 \$55 00	48 24 16 16 6 6 16 80	\$2,640,00 \$1,920,00 \$880,00 \$320,00 \$450,00 \$450,00 \$880,00 \$4,400,00	Labor Labor Labor Labor Labor Labor Labor Labor Labor	
Berm and liner installation supervision supervision Knockout tank installation, technician Transplinist leachate collection tank, laborer install blower w/ vanable speed process control, electrician install knockout tank liquid level controller, electrician soil pile construction, loader operator	hour hour hour hour hour hour	\$55 00 \$55 00 \$80 00 \$55 00 \$20 00 \$75 00 \$75 00 \$75 00 \$55 00	48 24 16 16 6 6 16	\$2,640.00 \$1,920.00 \$880.00 \$320.00 \$450.00 \$450.00 \$880.00	Labor Labor Labor Labor Labor Labor Labor	

Installation Cost:

\$29,973.09

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## **TEMPORARY BIOPILE SAMPLE COST ESTIMATES (Continued)**

# **Operation and Maintenance Costs**

**Temporary Biopile Installation** 10.00

Task

Total O&M Cost:

\$21,770.00

<u> </u>	abor Cost	Per Cell			
	Unit	Unit Cost	Project Life	Hours	Total Cost
	Hour	00.000	1		

			· · · · · · · · · · · · · · · · · · ·	110413	Total Cost
Initial project meeting: supervisor	Hour	\$80.00		4	\$320.00
Initial project meeting: technician	Hour	\$55.00		8	\$440.00
System startup: technician	Hour	\$55.00		12	\$660.00
System startup: supervisor	Hour	\$80.00		4	\$320.00
Weekly inspection (16 hr/mon): technician	Hour	\$55.00	4	64	\$3,520.00
Record keeping: supervisor (4 hr/mon)	Hour	\$80.00	4	16	\$1,280.00
Grounds keeping (8 hr/mon): laborer	Hour	\$20.00	4	32	
Admin support (8 hr/mon): clerical	Hour	\$25.00		32	\$640.00
Respiration Test(16 hr/test): technician	Hour	\$55.00	4	32	\$800.00
Soil Sampling (6 hr/event): technician	Hour	\$55.00			\$1,760.00
Site closeout: loader operator	Hour	\$55.00	4	32	\$1,760.00
Site closeout: technician				16	\$880.00
Site closeout: supervisor	Hour	\$55.00		48	\$2,640.00
	Hour	\$80.00		24	\$1,920.00

O&M Labor Cost Subtotal Per Cell

Soil Sampling: 2	/cycle/cell
Soil Gas Sampling: 4	/cycle/cell
<b>Respiration Test: 2</b>	/cycle/cell

**Total Analysis Cost** 

\$4,830.00

\$16,940.00

Sampling	Cost Per Cell	for 1	Cycle
			-,

Item	Unit	Unit Cost	Quantity	Cost/Event	Cost/Cycle
Analysis: soil fuel scan	event	\$240.00	5	\$1,200,00	\$2,400.00
Analysis: soil BTEX	event	\$95.00	5	\$475.00	\$950.00
Analysis: % moisture	event	\$24.00	5	\$120.00	\$950.00
Analysis: heterotrophic plate count	event	\$30.00	5	\$150.00	\$300.00
Anaylsis: soil pH	event	\$18.00	5	\$90.00	\$180.00
Analysis: soil gas	event	\$190.00	1	\$190.00	\$760.00
Analysis Subtotal Per Cell for 1 Cycle		<u> </u>			\$4,830.00

Subsequent Biopile Construction Cost

\$0.00

## **Biopile System Cost Summary**

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Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96		
Total Volume to be Tr	reated	500 yd <sup>3</sup>	
Number of Cells		1 cell(s)	
Volume of Cell Size		500 yd <sup>3</sup> /cell	
Number of Cycles Expected Project Life,		1 cycles 4 months	
Total Temporary Bio	pile Installation	Cost	\$29,973.09
Total O&M Cost			\$21,770.00
Present V. Inflation Rate	alue of Annual (	Costs 0.035	
Inflation-Adjusted A	verage Annual (	Costs (Out-Years)	NA
TOTAL COST	ESTIMATE		\$51,744
Unit Cost (\$/yd	l <sup>3</sup> )		\$103.49
Comments:			

	Biopile Sys	stem Cost Sumn	nary
Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96		
Total Volume to be Tr Number of Cells Volume of Cell Size Number of Cycles Expected Project Life		1000 yd <sup>3</sup> 1 cell(s) 500 yd <sup>3</sup> /cell 2 cycles 8 months	
Total Temporary Bio	opile Installatior	n Cost	\$29,973.09
Total O&M Cost			\$45,610.00
Present V Inflation Rate	alue of Annual	0.035	
Inflation-Adjusted A	verage Annual	Costs (Out-Years)	NA
TOTAL COST	ESTIMATE		\$75,584
Unit Cost (\$/yc	1 <sup>3</sup> )		\$75.58
Comments:			

	Biopile Sy	stem Co	ost Summa	ry
Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96			
Total Volume to be Tr Number of Cells Volume of Cell Size Number of Cycles Expected Project Life		2000 yd <sup>3</sup> 1 cel 500 yd <sup>3</sup> 4 cyc 16 mo	l(s) /cell :les	
Total Temporary Bio	pile Installatio	n Cost		\$29,973.09
Total O&M Cost				\$93,290.00
<i>Present V</i> Inflation Rate	alue of Annual	Costs 0.035		
Inflation-Adjusted A	verage Annual	Costs (Out	-Years)	\$96,555.15
TOTAL COST	ESTIMATE			\$126,529
Unit Cost (\$/yd	1 <sup>3</sup> )			\$63.26
Comments:				

94

	Biopile Sy	stem Co	ost Summary	
Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96			
Total Volume to be Ta Number of Cells Volume of Cell Size Number of Cycles Expected Project Life		2000 yd 2 ce 500 yd 2 cy 8 mo	ell(s) I <sup>3</sup> /cell	
Total Temporary Bi	opile Installatio	n Cost		\$50,558.87
Total O&M Cost				\$88,850.00
Present N Inflation Rate Inflation-Adjusted A	/alue of Annua	0.035	ut-Years)	NA
TOTAL COST	ESTIMATE	E		\$139,409
Unit Cost (\$/y	d <sup>3</sup> )			\$69.70

Comments:

**Biopile System Cost Summary** 

Project Name: Sample Test Location: **Test Site** Date of Estimate 6/9/96 Total Volume to be Treated 5000 yd<sup>3</sup> Number of Cells 1 cell(s) Volume of Cell Size 500 yd<sup>3</sup>/cell Number of Cycles 10 cycles Expected Project Life, mont 40 months Total Temporary Biopile Installation Cost \$29,973.09 Total O&M Cost \$236,330.00 Present Value of Annual Costs Inflation Rate 0.035 Inflation-Adjusted Average Annual Costs (Out-Year \$253,262.48 TOTAL COST ESTIMATE \$283,236 Unit Cost (\$/yd<sup>3</sup>) \$56.65

Comments:

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# **Biopile System Cost Summary**

Project Name: Sample Test Location: Test Site Date of Estimate 6/9/96	
Total Volume to be Treated5000 yd3Number of Cells1 cell(s)Volume of Cell Size750 yd3/cellNumber of Cycles7 cyclesExpected Project Life, mont28 months	
Total Temporary Biopile Installation Cost	\$29,973.09
Total O&M Cost	\$164,810.00
Present Value of Annual Costs Inflation Rate 0.035	
Inflation-Adjusted Average Annual Costs (Out-Year	\$173,563.47
TOTAL COST ESTIMATE	\$203,537
Unit Cost (\$/yd³)	\$40.71
Comments:	

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# **Biopile System Cost Summary**

Project Name:	Sample Test		
Location:	Test Site		
Date of Estimate:	6/9/96		
		_	
Total Volume to be Tr	eated 5000	yd <sup>3</sup>	
Number of Cells		cell(s)	
Volume of Cell Size	500	yd <sup>3</sup> /cell	
Number of Cycles	5	cycles	
Expected Project Life	months 20	months	
Total Temporary Bio	pile Installation Cost		\$50,558.87
Total O&M Cost			\$224,780.00
	alue of Annual Costs		
Inflation Rate	0.035		
Inflation-Adjusted Av	/erage Annual Costs (	Out-Years)	\$232,647.30
	<		
TOTAL COST	ESTIMATE		\$283,207
	9		
Unit Cost (\$/yd	3)		\$56.64
Comments:			

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## **Biopile System Cost Summary**

Project Name:	Sample Test		
Location:	Test Site		
Date of Estimate:	6/9/96		
		2	
Total Volume to be Tr	reated	5000 yd <sup>3</sup>	
Number of Cells		2 cell(s)	
Volume of Cell Size		750 yd <sup>3</sup> /cell	
Number of Cycles		4 cycles	
Expected Project Life,	, months	16 months	
Total Temporary Bio	opile Installation	n Cost	\$50,558.87
Total O&M Cost			\$174,640.00
Present V Inflation Rate	alue of Annual	<b>Costs</b> 0.035	
Inflation-Adjusted A	verage Annual	Costs (Out-Years)	\$180,752.40
TOTAL COST	ESTIMATE		\$231,312
Unit Cost (\$/yd	l <sup>3</sup> )		\$46.26
Comments:			

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## PERMANENT BIOPILE SAMPLE COST ESTIMATES

## Permanent Biopile Installation

		. miopi		canation			
Project Name: Sample Test							
	: Test Site						
Date of Estimate			Unit Ce	Il Dimensions: 5	50 ft x 60 ft		
Total Volume to be Treated	: 500	yd³	OVA Re	ntel 🛛 🛛			
Number of Cells		Cell(s)	OVA Pa	rchase			
Volume of Cell Size		yd'/cell	None				
Projected Cycle Length		months	Injection		1		
Total Project Length		months	Extractio	••• 🛛			
Total Permanent Biopile Installation Costs		76,725			•		
item	Unit	Unit Cost		y Total Cost	Comments	Vendor	
60-mil geotextile HDPE liner 55' x 65'	each	\$9,435.54	_	\$9,435.54	installed w compacted soil base	Racer	
2-inch flexible, slotted PVC 2-inch end caps for drainage pipe	ft	\$0.18	90	\$16.20		Discount Drainage	
2-inch schedule 40 PVC pipe	each	\$10.22	3	\$30.66		U.S. Plastics	
2"- 4" rubber reducer	20 ft each	\$69.00	1.5	\$103.50		U.S. Plastics	
2-inch brass gate valves	each	\$417.33	3	\$17.67 \$1,251.99		Discount Drainage	
2-inch galvanized steel pipe SCH 40	10 ft	\$25.20		\$25.20		Pipe-Valves Inc.	
2-inch PVC slip tee	each	\$2.31	2	\$4.62	<u> </u>	Pipe-Valves Inc.	
2-inch PVC slip elbow (will vary depending on site lavout)	each	\$2.51	2	\$5.02		U.S. Plastics	
2-inch SCH 40 PVC pipe (adjust as dictated by proximity of pump)	20 ft	\$23.48	8	\$187.81	<u>_</u>	U.S. Plastics	
20-gal heavy duty PVC water knockout tank	each	\$151.75	1	\$151.75		U.S. Plastics	
point switch/alarm controller (level control)	each	\$300.00	1	\$300.00		Cole-Parmer	
fiber-optic sensor (for level controller) 1-inch SCH 40 PVC pipe (adjust as required to plumb)	each	\$215.00	1	\$215.00		Cole-Parmer	
500-gal water collection tank	20 ft	\$10.99	1	\$10.99		U.S. Plastics	
1-1/2 hp rotary centrifugal vacuum pump	each each	\$860.00	1	\$860.00		Cole-Parmer	
4" thick concrete pad (10' x 12') for storage shed	sq.ft	\$821.00 \$6.00	1	\$821.00		Grainger	
storage shed to house pump and materials	each	\$249.00	120	\$720.00		General Maintenance & Eng. Co	
55-gai granular activated carbon drum	each	\$496.00	2	\$249.00 \$992.00	······	Grainger	
misc. PVC fittings to connect knockout tank, pump and carbon drums		\$250.00		\$250.00		Carbtrol Corp	
20-mil 70-ft x 75-ft plastic sheeting (HDPE or qualified substitute)	each	\$200.00		\$200.00		Racer	
5/16-inch x 3-inch hex head sheet metal screw	each	\$0.10	150	\$15.00		Dotter fastener	
nylon rope, 5/16"	100 ft	\$19.23	4	\$76.92		Grainger	
Urea	50 lb bag		58	\$522.00		Lesco	
Ammonium Phosphate, Dibasic Potassium Sulfate	50 lb bag		8	\$356.00		Ashland Chemical	
Superphosphate	50 lb bag			\$0.00		Lesco	
water hoses with nozzles	50 lb bag			\$0.00		Lesco	
scoop for nutrient addition	each each	\$33.69 \$5.00	4	\$134.76		Columbus Hardware	
scale to measure nutrients	each	\$68.05	1	\$5.00		K-Mart	
2 1/2-gal bucket for holding/distributing nutrients	each	\$5.45	1	\$68.05 \$5.45		Grainger	
1/4-inch OD nylon tubing for monitoring points-green	50ft	\$19.25	3	\$57.75		Cole-Parmer	
1/4-inch OD nylon tubing for monitoring points-red	50ft	\$19.25	3	\$57.75		Cole-Parmer Cole-Parmer	
1/4-inch OD nylon tubing for monitoring points-blue	50ft	\$19.25	3	\$57.75		Cole-Parmer	
Thermocouple display, hand held	each	\$65.00	1	\$65.00		Cole-Parmer	
K-type thermocouple wire, 24-gauge K-type thermocouple plug	100 ft roll	\$48.50	1	\$48.50		Cole-Parmer	
NPT 3/8 X 1/4 tube fitting	each	\$3.18	10	\$31.80		LH Marshali	
female quick-connect	each	\$1.31	10	\$13.10		Forberg Scientific	
gravel for strainer	each 50 lb	\$12.10	13	\$157.30		Forberg Scientific	
suction strainer 3/4"	each	\$20.00 \$9.49	1	\$20.00		estimate aquarium supply	
3/8" thread tap	each	\$6.32	10	\$94.90		Grainger	
male quick connect X 1/4 " tubing	each	\$11.88	13	\$63.20 \$154.44		Columbus Hardware	
quick-connect protectors	each	\$5.01	13	\$65.13		Forberg Scientific	
shovel	each	\$20.95	2	\$41.90		Forberg Scientific	
1/3 HP sump pump	each	\$131.78	1	\$131.78		J.S. Plastics	
1/3 HP transfer pump	each	\$240.50	1	\$240.50		Grainger	
Aeration/Leachate Collection Channel 8" Structural Slab on grade	each	\$3,294.58	1	\$3,294.58		Racer	
Gas Tech 3250X O2/CO2		\$33,499.01	1	\$33,499.01		Racer	
Gas Tech GT105 02/CD2 Gas Tech GT105 02/TPH	each	\$3,700.00	1	\$3,700.00		Control Analytics	
OVA	each	\$1,548 75	1	\$1,548.75		Control Analytics	
Soil Compaction: technician	weekly	\$477 00	3	\$1,431.00		lazco	
Berm and liner installation: technician	hour	\$55.00	16	\$880.00	Labor		
Berm and liner instalation supervision supervisor	hour	\$55.00	48	\$2,640.00	Labor		
Knockout tank installation: technician	hour	\$80.00 \$55.00	24	\$1,920.00	Labor		
Transpt/insti leachate collection tank, laborer	hour	\$20 00	16 16	\$880.00	Labor		
	hour	\$75.00	6	\$320.00	Labor		
install blower w/ variable speed process control electrician				\$450.00	Labor		
install knockout tank liquid level controller electrician	hour	\$75.00	6	\$450 001	char		
install knockout tank liquid level controller electrician soil pile construction: loader operator		\$75.00 \$55.00	6 16	\$450.00 \$880.00	Labor		
Install blower w/ variable speed process control electrician Install knockout tank liquid level controller electrician soil pile construction: loader operator soil pile construction: technician	hour	\$75.00 \$55.00 \$55.00	6 16 80	\$880.00	Labor		
Install knockout tank liquid level controller electrician soil pile construction: loader operator	hour hour	\$55.00	16				

Installation Cost

\$76,725.27

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## **Operation and Maintenance Costs**

Permanent Biopile Installation

Total O&M Cost:

\$21,770.00

Task	Unit	Unit Cost	Project Life	Hours	Total Cost
Initial project meeting: supervisor	Hour	\$80.00	i — — †	4	\$320.00
Initial project meeting: technician	Hour	\$55.00		8	\$440.00
System startup: technician	Hour	\$55.00		12	\$660.00
System startup: supervisor	Hour	\$80.00	├────┼	4	\$320.00
Weekly inspection (16 hr/mon): technician	Hour	\$55.00	4	64	
Record keeping: supervisor (4 hr/mon)	Hour	\$80.00	4	16	\$3,520.00
Grounds keeping (8 hr/mon): laborer	Hour	\$20.00	4	32	\$1,280.00
Admin support (8 hr/mon): clerical	Hour	\$25.00	4		\$640.00
Respiration Test(16 hr/test): technician	Hour	\$55.00		32	\$800.00
Soil Sampling (6 hr/event): technician	Hour	\$55.00	4	32	\$1,760.00
Site closeout: loader operator	Hour		4	32	\$1,760.00
Site closeout: technician		\$55.00		16	\$880.00
Site closeout: supervisor	Hour	\$55.00		48	\$2,640.00
O&M Labor Cost Subtotal Per Cell	Hour	\$80.00		24	\$1,920.00

por Cost Subtotal Per Cell

\$16,940.00

Soil Sampling: 2 /cycle/cell Soil Gas Sampling: 4 /cycle/cell Respiration Test: 2 /cycle/cell

**Total Analysis Cost** 

\$4,830.00

Sampling Cost Per Cell for 1 Cycle						
Item	Unit	Unit Cost	Quantity	Cost/Event	Cost/Cycle	
Analysis: soil fuel scan	event	\$240.00	5	\$1,200.00	\$2,400.00	
Analysis: soil BTEX	event	\$95.00	5	\$475.00	\$950.00	
Analysis: % moisture	event	\$24.00	5	\$120.00	\$240.00	
Analysis: heterotrophic plate count	event	\$30.00	5	\$150.00	\$300.00	
Anaylsis: soil pH	event	\$18.00	5	\$90.00	\$180.00	
Analysis: soil gas	event	\$190.00	1	\$190.00	\$760.00	
Analysis Subtotal Per Cell for 1 Cycle					\$700.00	

Analysis Subtotal Per Cell for 1 Cycle

\$4,830.00

Subsequent Biopile Construction Cost

\$0.00

## H.2 PERMANENT BIOPILE SAMPLE COST ESTIMATES (Continued)

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# **Biopile System Cost Summary**

Project Name:	Sample Test			
Location:	Test Site			
Date of Estimate:	6/9/96			
Total Volume to be Ti	reated	500 yd <sup>3</sup>		
Number of Cells		1 cell(s)		
Volume of Cell Size		500 yd <sup>3</sup> /cell		
Number of Cycles		1 cycles		
Expected Project Life	, months	4 months		
Total Permanent Biopile Installation Cost Total O&M Cost		\$76,725.27		
			\$21,770.00	
Present V. Inflation Rate	alue of Annual C	<b>Costs</b> 0.035		
Inflation-Adjusted Average Annual Costs (Out-Years)			NA	
TOTAL COST ESTIMATE			\$98,496	
Unit Cost (\$/yd³)			\$196.99	
Comments:				

102

H.2 PERMANENT BIOPILE SAMPLE COST ESTIMATES (Continued)

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Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96		
Total Volume to be T Number of Cells Volume of Cell Size Number of Cycles Expected Project Life		5000 yd <sup>3</sup> 1 cell(s) 500 yd <sup>3</sup> /cell 10 cycles 40 months	
Total Permanent Bi	opile Installation	Cost	\$76,725.27
Total O&M Cost			\$237,761.00
Present N Inflation Rate	/alue of Annual (	Costs 0.035	
Inflation-Adjusted Average Annual Costs (Out-Years)			\$254,796.01
TOTAL COST ESTIMATE			\$331,522
Unit Cost (\$/yo	d <sup>3</sup> )		\$66.30
<b>C</b>			

Comments:

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`\_ **Biopile System Cost Summary** Project Name: Sample Test Location: **Test Site** Date of Estimate: 6/9/96 Total Volume to be Treated 5000 yd<sup>3</sup> Number of Cells 1 cell(s) Volume of Cell Size 750 yd<sup>3</sup>/cell Number of Cycles 7 cycles Expected Project Life, months 28 months Total Permanent Biopile Installation Cost \$76,725.27 Total O&M Cost \$166,241.00 Present Value of Annual Costs Inflation Rate 0.035 Inflation-Adjusted Average Annual Costs (Out-Years) \$175,070.48 TOTAL COST ESTIMATE \$251,796 Unit Cost (\$/yd<sup>3</sup>) \$50.36 Comments:

104

	Biopile Sy	stem Cost Summa	ry
Project Name: Location:	Sample Test Test Site		
Date of Estimate:	6/9/96		
Total Volume to be T Number of Cells	reated	10000 yd <sup>3</sup> 1 cell(s)	
Volume of Cell Size Number of Cycles		500 yd <sup>3</sup> /cell 20 cycles	
Expected Project Life	e, months	80 months	
Total O&M Cost			\$476,161.00
Present Inflation Rate	Value of Annua	/ Costs 0.035	
Inflation-Adjusted	Average Annual	Costs (Out-Years)	\$538,014.9
TOTAL COST	ESTIMATE	E	\$614,741
Unit Cost (\$/y	d³)		\$61.47

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# **Biopile System Cost Summary**

Project Name:SampleLocation:Test SDate of Estimate:6/9/9	ite	
Total Volume to be Treated Number of Cells Volume of Cell Size	10000 yd <sup>3</sup> 2 cell(s) 500 yd <sup>3</sup> /cell	
Number of Cycles Expected Project Life, months	10 cycles	
Total Permanent Biopile Inst	tallation Cost	\$141,875.30
Total O&M Cost		\$452,761.00
<b>Present Value of A</b> Inflation Rate	Annual Costs	
Inflation-Adjusted Average A	nnual Costs (Out-Years)	\$485,200.25
TOTAL COST ESTIN	MATE	\$627,076
Unit Cost (\$/yd³)		\$62.71
Comments:		

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	Dioplie Sy	vstem Cost Summary	
Project Name:	Sample Test		
Location:	Test Site		
Date of Estimate:	6/9/96		
Total Volume to be T	reated	20000 yd <sup>3</sup>	
Number of Cells		2 cell(s)	
Volume of Cell Size		500 yd <sup>3</sup> /cell	
Number of Cycles		20 cycles	
Expected Project Life	, months	80 months	
Total Permanent Bio	opile Installatio	n Cost	\$141,875.30
Total O&M Cost			\$905,861.00
<b>Present V</b> Inflation Rate	alue of Annual	0.035	
Inflation-Adjusted A	verage Annual	Costs (Out-Years)	\$1,023,533.48
TOTAL COST	ESTIMATE	E	\$1,165,409
Unit Cost (\$/yd	l <sup>3</sup> )		\$58.27
Comments:			

Biopile System Cost Summary

.\_

Project Name: Location: Date of Estimate:	Sample Test Test Site 6/9/96		
Total Volume to be Tre	eated	20000 yd <sup>3</sup>	
Number of Cells		4 cell(s)	
Volume of Cell Size		500 yd <sup>3</sup> /cell	
Number of Cycles		10 cycles	
Expected Project Life,	months	40 months	
Total Permanent Bio	pile Installatio	n Cost	\$274,386.35
Total O&M Cost			\$909,545.00
Present Va Inflation Rate	alue of Annual	0.035	
Inflation-Adjusted Av	/erage Annual	Costs (Out-Years)	\$974,711.74
TOTAL COST	ESTIMATE	E .	\$1,249,099
Unit Cost (\$/yd	<sup>3</sup> )		\$62.45
Comments:			

H.2

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Project Name:

PERMANENT BIOPILE SAMPLE COST ESTIMATES (Continued)

Sample Test

# Biopile System Cost Summary

Location:	Test Site		
Date of Estimate:	6/9/96		
Total Volume to be T Number of Cells Volume of Cell Size Number of Cycles Expected Project Life		40000 yd <sup>3</sup> 4 cell(s) 750 yd <sup>3</sup> /cell 14 cycles 56 months	
Total Permanent Bic	opile Installati	on Cost	\$274,386.35
Total O&M Cost			\$1,264,789.00
Present V Inflation Rate	alue of Annua	0.035	
Inflation-Adjusted A	verage Annua	l Costs (Out-Years)	\$1,379,399.71
TOTAL COST	ESTIMATI	E	\$1,653,787
Unit Cost (\$/yd	<sup>3</sup> )		\$41.34
<b>Commun</b> (			

Comments:

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### H.3 BIOPILE COST ESTIMATOR<sup>©</sup> INSTRUCTION GUIDE

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Prepared by F. Michael von Fahnestock and Sam W. Yoon, Battelle, for Naval Facilities Engineering Service Center, May 29, 1996.

This instruction guide gives a visual overview of the Biopile Cost Estimator<sup>©</sup> software program used to generate the preceding cost sheets for the various biopile design scenarios. This program can be obtained from NFESC, located at 1100 23rd Avenue, Port Hueneme, CA, 93043-4301.

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# Main Menu

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The main menu allows access to all input and report screens.

The root for the	1
BIOPILE COST ESTIMATOR	
Version 1.21	No.
Battelle Memorial Institute	STATES AND
- Biopile Parameters	COLO MARINO CONTRA
BIOPILE COST ESTIMATOR Version 1.21 © Battelle Memorial Institute  Biopile Parameters Biopile Costs Biopile Costs	ALL COLORD STATEMENTS AND A DESCRIPTION
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Propared by Environmental Restoration Department of	
Battelle Memorial Institute for the Naval Facilities Engineering Service Center.	

# **Biopile Design Parameters**

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Click the **DESIGN PARAMETERS** button to reach the Biopile Parameters screen. Use this screen to input the design variables. Recommended default values are given in brackets. This window initially displays the values last saved in the program. Use the mouse or the tab key to move to the desired input field.

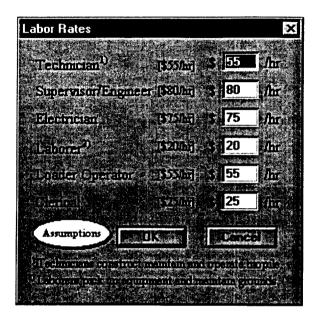
Click the **OK** button once all variables have been specified. The program will calculate the installation and O&M costs and will then automatically display the Installation Cost Table.

Click the **ASSUMPTIONS** button to list the general design assumptions used with this cost estimation tool.

Biopile Parameters	
Sample Test	i in minimu
Test Site	6/9/96
Villagens Containte Reis Soll	
Sales with Values of Basical 500 7	e suissinging Zayteri 2 ayel
	Sols car sampling country. 4 oyau
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• E-mennery • Q Reformation	Assumptions
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### **Labor Rates**

The hourly rates for various labor categories can be adjusted to make the cost estimate site specific. To list the labor rate assumptions, click the **ASSUMPTIONS** button. Click the **OK** button to accept the labor rates entered and to forward to the Installation Cost Screen.



Note: Labor rates shown above are for this example case only. Actual rates should be entered on a site-specific basis.

# **Installation Cost Screen**

This screen lists installation costs for the biopile design selected on a 'per cell' basis. The table gives a line item cost breakout of all materials and equipment required.

Click the **MAIN** button to return to the main menu. To change design specifications, select the **DESIGN PARAMETERS** button from the main menu and reenter values as desired.

Click the O&M COSTS button to view the O&M Cost Screen.

Click the SUMMARY COST button to view the overall biopile design cost.

Click the **PRINT** button to produce a hard copy of the Installation Cost Screen.

				hadaaliitti k	lland bos 71 tiba.	1
	Perma	anent	Biopil	e Inst	allation	
Calific Print 2						
	Project Name:	•	est			
	Location:					
Children States	Date of Estimate:	6/9/9	5	Unit Cell Dimensions:		50 ft x 60 ft
	Total Volume to be Treated:	5000	yd³	OVA Rece		
COMPACT NAMES OF A DESCRIPTION	Number of Cells:	1	Cell(s)	OVA Pare	lase T	
	Volume of Cell Size:	500	yd'/cell	None 🌾	States and T	
	Projected Cycle Length:	4	months	Injection ?		
	Total Project Length:	40	months	ante fin ante fingente te benefenten fite	Xi	
Total Perma	nent Biopile Installation Costs:	\$7	5,725			1
	tem	Unit	Unit Cost	Quantity	Total Cost	Comments
60-mil geotextile HDPE liner 55'	x 65'	each	\$9,435.54	1	\$9,435.54	installed w/ compacted so
2-inch flexible, slotted PVC		ft	\$0.18	90	\$16.20	
2-inch end caps for drainage pip	e	each	\$10.22	3	\$30.66	
2-inch schedule 40 PVC pipe		20 ft	\$69.00	1.5	\$103.50	
2*- 4* rubber reducer		each	\$5.89	3	\$17.67	
2-inch brass gate valves		each	\$417.33	3	\$1,251.99	
2-inch galvanized steel pipe SCH	140	10 ft	\$25.20	1	\$25.20	
2-inch PVC slip tee		each	\$2.31	2	\$4.62	
2-inch PVC slip elbow (will vary i	depending on site layout)	each	\$2.51	2	\$5.02	
2-inch SCH 40 PVC pipe (adjust	as dictated by proximiy of pump)	20 ft	\$23.48	8	\$187.81	
20-gal heavy duty PVC water kn		each	\$151.75	_ 1	\$151.75	
point switch/alarm controller (lev		each	\$300.00	1	\$300.00	
fiber-optic sensor (for level contro		each 20 ft	\$215.00	1	\$215.00	
1-inch SCH 40 PVC pipe (adjust as required to plumb)			\$10.99	1	\$10.99	
500-gal water collection tank			\$860.00	1	\$860.00	
1-1/2 hp rotary centrifugal vacuum pump			\$821.00	1	\$821.00	
4" thick concrete pad (10 x 12) for storage shed			\$6.00	120	\$720.00	
storage shed to house pump and		each	\$249.00	1,	\$249 00	
55-gal granular activated carbon	drum	each	\$496.00	2	\$992.00	

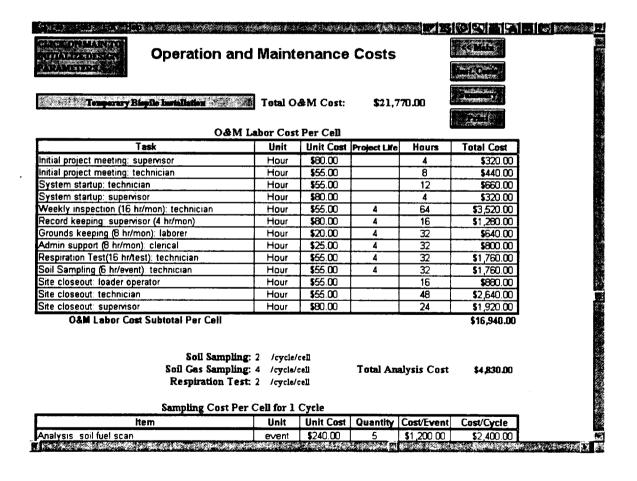
### **O&M Cost Screen**

View the O&M Cost Screen by clicking the O&M COSTS button from the main menu, or by clicking the O&M COSTS button in the Installation Cost Screen, or the Summary Screen.

The O&M Cost Screen lists the total O&M cost estimate and sampling costs per cell. This screen also gives the total biopile cell reconstruction cost for sites requiring multiple cycles.

Return to the main menu or go to the Installation Cost or Summary Cost Screens by clicking the appropriate buttons.

Click **PRINT** to produce a hard copy of the O&M Cost Screen.



## **Cost Summary Screen**

The Cost Summary Screen lists the total costs estimate for the specified design and gives the installation and O&M cost subtotals. This screen also lists the unit cost, dollars per cubic yard, of the specified design. An annual inflation factor adjusts the total cost for inflation on projects lasting more than one year.

Along with design costs, the Cost Summary Screen lists the specified design parameters and the project title, location, and date of the estimate.

Click MAIN to return to the main menu and PRINT to produce a hard copy of the Cost Summary Screen. Clicking the INSTALLATION COST or the O&M COSTS buttons will forward the program to those respective screens.

Biopile Syste	em Cost Summa	
Project Name:       Sample Test         Location:       Test Site         Date of Estimate:       6/9/96         Total Volume to be Treated         Number of Cells         Volume of Cell Size         Number of Cycles	500 yd <sup>o</sup> 1 cell(s) 500 yd <sup>o</sup> /cell 1 cycles	
Expected Project Life, months	1 cycles 4 months	
Total Temporary Biopile Installation	Cost	\$29,973.09
Total O&M Cost		\$21,770.00
Present Value of Annual C Inflation Rate	<b>380</b>	
Inflation-Adjusted Average Annual C	osts (Out-Y <b>ae</b> rs)	NA
TOTAL COST ESTIMATI	E	\$51,744
Unit Cost (\$/yd³)		\$103.49

#### **APPENDIX I**

#### RESPIRATION TEST DATA CALCULATION AND WORKSHEET

The respiration test is performed to obtain data for calculating the TPH degradation rates in the biopile soil. In the respiration test, Q levels are measured in soil gas sampled from the monitoring points installed in various locations of the biopile. Readings generally are taken until oxygen concentrations drop below 7% or until the Q concentration no longer decreases. If Q decreases rapidly, more frequent readings will be necessary than if Q decreases slowly. To determine the oxygen utilization rate, oxygen percent is plotted against time. The slope of this line is referred to as the oxygen utilization rate and is reported as change of oxygen percent per day.

If low oxygen levels become a limiting factor forbiodegradation, the slope of the line will level off and no longer be indicative of oxygen consumption relative to TPH degradation. In this case, only the linear portion of the curve, generally limited to data points at or above 12% Q, will be used to calculate biodegradation rates.

The stoichiometric relationship between oxygen consumption and TPH degradation using hexane as a representative compound is shown in Equation 1:

$$C_6H_{14} + 9.5O_2 \rightarrow 6CO_2 + 7H_2O$$
 (1)

Using this equation, the biodegradation rate in terms of milligrams of hexane-equivalent per kilogram of soil per day can be estimated.

The first step in this calculation (Equation 2) is to convert the percentage of Q in soil gas to the actual amount in the form of mg Q/kg of soil. Properties of both oxygen and the soil consistency in the biopile are used to calculate this value. One mole of air at a temperature of 300 K would occupy a volume of 24.6 L. Assuming a soil-gas oxygen concentration such as that of ambient air (20.9%), only 5.14 L of the 24.6 L/mole soil gas would be occupied by  $O_2$ .

24.6 L/mole of soil gas 
$$\times$$
 20.9% O<sub>2</sub> = 5.14 L of O<sub>2</sub>/mole of soil gas (2)

This value would vary according to the reported oxygen concentration. As shown in Equation 3, for example, an oxygen concentration of 15% would result in 3.69 L Q/mole of air instead of 5.14 L Q/mole of soil gas.

24.6 L/mole of soil gas 
$$\times$$
 15% O<sub>2</sub> = 3.69 L of O<sub>2</sub>/mole of soil gas (3)

To determine the mass of the 5.14 L Q/mole of soil gas, the density of Q must be used. Because 1 mole of  $O_2$  would have a mass of 32 g and occupy a volume of 24.6 L, the density of Qwould be 1,300 mg/L (Equations 4 and 5).

$$32 g \div 24.6 L \text{ of } O_2 = 1.300 g/L \text{ of } O_2$$
 (4)

$$1.300 \text{ g/L of } O_2 \times 1000 \text{ mg/g} = 1,300 \text{ mg/L of } O_2$$
 (5)

This value multiplied by 5.14 L/mole soil gas would yield 6,682 mg Q/mole soil gas (Equation 6) or 271.6 mg Q/L soil gas (Equation 7).

1,300 mg/L of 
$$O_2 \times 5.14$$
 L of  $O_2$ /mole of soil gas = 6,682 mg  $Q_2$ /mole of soil gas (6)

$$6,682 \text{ mg } O_2/\text{mole of soil gas} \div 24.6 \text{ L/mole of soil gas} = 271.6 \text{ mg } Q/L \text{ soil gas}$$
 (7)

Once this relationship has been established, it must be determined what quantity of oxygen would exist

in the void volume of 1 kg of soil. Assuming a soil density of  $2,400 \text{ lb/yd}(1,424 \text{ kg/m}^3)$ , Equation 8 shows that 1 kg of soil would occupy a volume of 0.702 L.

$$(1,424 \text{ m}^3/\text{kg}) \times 1,000 \text{ L/m}^3 = 0.702 \text{ L/kg}$$
 (8)

Assuming a void volume of 30% in the soil, the volume of 1 kg of soil that would be occupied by soil gas is 0.21 L (Equation 9).

$$0.702 \text{ L/kg} \times 30\%$$
 void volume =  $0.21 \text{ L}$  soil gas/kg soil (9)

Using the conversion factor from Equation 7 of 271.6 mg Q/L air, it can be calculated in Equation 10 that 57.04 mg of  $O_2$  would be present in 1 kg of soil at an Q concentration of 20.9%.

$$0.21 \text{ L soil gas/kg soil} \times 271.6 \text{ mg } O_2/\text{L soil gas} = 57.04 \text{ mg } O_2/\text{kg soil}$$
(10)

Once the change in mass of Q has been calculated, Equation 1 can be used to determine the mass of hydrocarbons that theoretically would be degraded. The equation yields a hydrocarbon-to-oxygen mass ratio of 1:3.5 to oxidize hexane. Therefore, if a decrease of  $50 \text{mg O}_2/\text{kg}$  soil were seen, then it could be assumed that 14.3 mg TPH/kg of soil had been degraded. As shown in Equation 11, the TPH degradation rate can be calculated from the Q degradation rate (mg Q/kg h) divided by 3.5, which is the Q-to-hydrocarbon mass ratio described above.

$$50 \text{ mg } O_2/\text{kg} \div 3.5 \text{ mg } O_2/\text{mg } \text{TPH} = 14.3 \text{ mg } \text{TPH}/\text{kg of soil}$$
 (11)

Figure I-1 is a completed example of a worksheet to convert respiration sampling data (%Qdecrease with time) to the TPH degradation rate. Figure I-2 is a blank TPH degradation worksheet that can be copied and used on site.

		TPH DEGRADATION RATE WORKSHEET		
<ol> <li>a) O<sub>2</sub> concentration reading at time of blower shutdown</li> <li>b) O<sub>2</sub> concentration reading nearest to and greater than 12%</li> <li>c) Change in O<sub>2</sub> concentration (Line 1a - Line 1b)</li> </ol>				
2.	a)	Elapsed time from shutdown to final Q reading	<u>hr</u>	
3.	Oxy a) b)	gen Utilization Rate Change in O2 concentration/elapsed time (Line 1c/Line 2a) Line 3a × 24	<u>%/hr</u> %/day	
Bas	ed on	the oxygen utilization rate, use the following equation to calculate degradation rate:		
		$K_{B} = -K_{O}AD_{O}C$		
		100		
whe				
		= degradation rate (mg/kg-day)		
	Ko	= oxygen utilization rate (%/day)		
		From Line 3b	<u>%/day</u>	
4.	А	= volume of air/kg soil (L/kg)		
	a)	Density of soil (if unknown assume a bulk density of 2,400 lb/yd)	<u>lb/yd³</u>	
	b)	Vol soil/kg soil: $(764.6 \text{ L/yd} \times 2.205 \text{ lb/kg}) \div \text{Line } 4a =$	L/kg	
	c)	Vol air/kg soil: Line $4b \times 0.30^* =$ * (assuming 30% soil porosity)	L/kg	
5.	$\mathbf{D}_{\mathrm{o}}$	= density of oxygen gas (mg/L)		
	a)	Size temperature: $^{\circ}C^{**} + 273 =$ ** (assume 27°C if unknown)	<u>K</u>	
	b)	Volume per mole: $0.08205 \times \text{Line 5a} =$	L/mole	
	c)	Mass O <sub>2</sub> per liter: 32,000 mg/mole ÷ Line 5b	mg/L	
6.	С	= mass ratio of hydrocarbon to oxygen required formineralization (1/3.5)	0.2857	
	ТР	<b>H Degradation Rate</b> = (Line 3b × Line 4c × Line 5c × Line 6) $\div$ 100	mg/kg-day	

Figure I-1. Example of a Completed TPH Degradation Rate Worksheet.

		TPH DEGRADATION RATE WORKSHEET	
1.	a) b) c)	<u>%</u> <u>%</u>	
2.	a)	Elapsed time from shutdown to final Q reading	<u>hr</u>
3.	Oxy a) b)	gen Utilization Rate Change in O2 concentration/elapsed time (Line 1c/Line 2a) Line 3a×24	<u>%/hr</u> <u>%/day</u>
Bas	ed on	the oxygen utilization rate, use the following equation to calculate degradation rate:	
		$\mathbf{K}_{\mathrm{B}} = \underline{-\mathbf{K}_{\mathrm{O}}\mathbf{A}\mathbf{D}_{\mathrm{O}}\mathbf{C}}$	
la		100	
wh		de sus detions note (mes /liss des.)	
	K <sub>B</sub>	= degradation rate (mg/kg-day)	
	Ko	= oxygen utilization rate (%/day)	07.71
		From Line 3b	<u>%/day</u>
4.	A	= volume of air/kg soil (L/kg)	
	a)	Density of soil (if unknown assume a bulk density of 2,400 $lb/yd$ )	<u>lb/yd³</u>
	b)	Vol soil/kg soil: $(764.6 \text{ L/yd} \times 2.205 \text{ lb/kg}) \div \text{Line } 4a =$	L/kg
	c)	Vol air/kg soil: Line $4b \times 0.30^* =$ * (assuming 30% soil porosity)	L/kg
5.	$\mathbf{D}_{\mathrm{o}}$	= density of oxygen gas (mg/L)	
	a)	Size temperature: $^{\circ}C^{**} + 273 =$ ** (assume 27°C if unknown)	<u>K</u>
	b)	Volume per mole: $0.08205 \times \text{Line 5a} =$	L/mole
	c)	Mass O <sub>2</sub> per liter: 32,000 mg/mole ÷ Line 5b	mg/L
6.	С	= mass ratio of hydrocarbon to oxygen required formineralization (1/3.5)	0.2857
	ТР	<b>H Degradation Rate</b> = (Line $3b \times Line 4c \times Line 5c \times Line 6) \div 100$	mg/kg-day

Figure I-2. TPH Degradation Rate Worksheet

#### APPENDIX J

#### **EXAMPLE DESIGN CALCULATIONS**

**J.1 Design Scenario.** There are 500 yc<sup>4</sup> (382 m<sup>3</sup>) of soil contaminated with 20,000 mg TPH per kg of soil to be treated. The soil has a 10% moisture content which is approximately 50% of field capacity. The soil is relatively uniform and has a minimal clay, silt, and organic content. Initial soil data show the pH to average 7.9. The total organic carbon content and N:P:K values are unknown at this point. Referring to the decision tree in Chapter 1, the biopile technology appears feasible, provided sufficient moisture and nutrients are added. Soil processing does not appear to be necessary.

#### **J.2 Calculating the Biopile Dimensions** . The following biopile dimensions are assumed:

Total soil volume to be processed = Desired biopile height Expected biopile slope at sides = 500 yd<sup>3</sup> (382 m<sup>3</sup>) = 5 to 6 ft (1.5 to 1.8 m) 1.25:1 side to height ratio

Figure J-1 shows an examplebiopile.

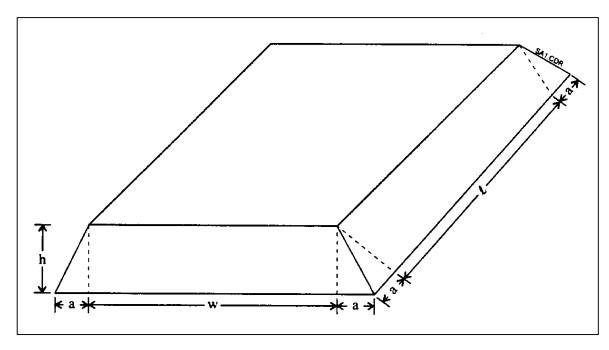


Figure J-1. Example Biopile Showing Approximate Relative Dimensions.

The pad can be sized based on the volume of soil to be processed and by assuming an average pile height and pile slope. The volume of the biopile is represented by equation (1) for the volume of aprismoid:

$$V = \frac{1}{6} h (B_1 + 4 M + B_2)$$
(1)

where:

V = volume of pile h =pile height area of lower base B<sub>1</sub> =  $B_2 =$ area of upper base area of biopile midsection M = The design volume is 500 yd = V height is 5 ft = h  $B_1 =$  $(l + 2a) (w + 2a) = lw + 2aw + 2al + 4a^{2}$  $B_2 =$ lw  $(l + 2(a/2)) (w + 2(a/2)) = lw + aw + al = a^{2}$ M = V  $(h/6) [(lw + 2aw + 2al + 4a^{2}) + lw + 4 (lw + aw + al + a^{2})]$ = V =  $(h/6) [(6 lw = 6aw + 6al + 8a^2)]$  $V = h (lw + aw + al + 1.33 a^2)$  $V/h = (l + a)w + (al + 1.33 a^2)$  $[V/h - (al + 1.33 a^2)]/(l + a)$ w =

Solve for a:

1.25 h			1	Assume side to height slope $= 1.25:1$
		h		
θ		si	nθ =	= h/1.25 h = 0.8
;	a		θ =	= 53.13°
		ta	unθ =	= h/a = 5/a
			a =	$= 5/\tan(53.13) = 3.75$ ft

To size pile, choose alength and then calculate overall pile width.

Assume total pile length = 60 ft = 1 + 2a60 ft l = 60 - 2a = 60 - 2 (3.75) $1 = 52.5 \, \text{ft}$ V = 13,500 ft<sup>3</sup> h = 5 ft 3.75 ft а = 1 52.5 ft =  $[6,750 \text{ ft}^2/5 \text{ ft} - (3.75 \text{ ft}) (52.5 \text{ ft}) + 1.33 (3.75 \text{ ft})^2]/(52.5 \text{ ft} + 3.75 \text{ ft})$ w = 44.8 ft w = total pile width 44.4 ft + 2 (3.75 ft) = 52 ft

biopile dimensions would be: 60 ft  $\times$  52 ft  $\times$  5 ft

The biopile dimensions can be calculated by selecting a new overall pad length and repeating the above calculations.

In general, the 50 ft  $\times$  60 ft pad area will be suitable forbiopile designs ranging in increments of 400 to 750 yd<sup>3</sup>. Smaller process batches may require padresizing using the above calculation steps. Larger volumes of soil can be processed using multiple 500-yd<sup>4</sup> pads. Generally the minimum pile height should be 3½ ft and the maximum pile height should be 8 ft.

**J.3** Calculating the Amou nt of Nutrients to Be Added . Figure J-2 is a completed example of a worksheet to complete the nutrient calculations presented below. Figure J-3 is a blankbiopile nutrient addition worksheet that can be copied and used on site.

Soil contamination level = 20,000/mg/kg

Total organic content unknown :			
	Assume C-content = $20,000 \text{ mg/kg}(0.8) = 16,000 \text{ mg/kg}$		
Ľ	Desired C:N:P = $100:15:1$		
	N needed = $(16,000 \text{ mg/kg}) \times (15/100) = 2,400 \text{ mg/kg}$		
	P needed = $(16,000 \text{ mg/kg}) \times (1/100) = 160 \text{ mg/kg}$		
Total kg soil:			
	Assume soil density = $2,400 \text{ lb/yd}$		
	oil volume = $500 \text{ yd}^3 = 382 \text{ m}^3$		
	$\begin{array}{llllllllllllllllllllllllllllllllllll$		
1	'otal soil mass (kg) = 1,200,000lb (0.45359 kg/lb) = 544,308 kg		
Total P-source needed :			
	P needed = $(544,308 \text{ kg soil}) (160 \text{ mg P/kg soil}) (1 \text{ kg/1,000,000 mg})$		
	= 87.1 kg P		
	= $(87.1 \text{ kg N}) (2.2046 \text{ lb/kg}) = 192 \text{ lb}$		
Р	P-source = diammonium phosphate, $(NH_3)_2HPO_4$		
	r r r v v v v		
	Diammonium phosphate (DAP) contains nitrogen as well as phosphorous. The nitrogen in this nutrient		
S	ource should be counted as part of the total N supplied.		
11	$p P/lb DAP HPO_4 = 0.24$		
	DAP needed = $(192 \text{ lb P}) \div (0.24 \text{ lb P/lb DAP})$		
	= 800 lb DAP		
Total N-source needed :			
Ν	N  needed = (544,308  kg soil) (2,400  mg N/kg soil) (1  kg/1,000,000 mg)		
	= 1,306  kg N (1.206 kg) (2.2046 lb (hg)) - 2.880 lb N		
	= $(1,306 \text{ kg}) (2.2046 \text{ lb/kg}) = 2,880 \text{ lb N}$		

	<b>BIOPILE NUTRIENT ADDITION WORKSHEET</b>
1.	Nutrient Source:       a. Nitrogen source (e.g. urea) weight fraction nitrogen (urea = 0.46) <sup>a</sup> b. Phosphorus source (e.g. diammonium phosphate) weight fraction phosphorus         c. Potassium source (e.g. potassium sulfate) weight fraction potassium
2.	Total organic carbon content in soil: mg/kg dry soil. Obtained from laboratory results. If unknown, calculate as below:a. Average concentration of hydrocarbon contamination in soil = mg/kg dry soilb. Average carbon content in contamination = line $2a \times 0.8$ = mg carbon/kg dry soil
3.	Desired C:N:P:K ratio. Determine by treatability tests, else use C:N:P:K = 100:15:1:1.
4.	Amount of nutrient to add per kg of dry soil. (If not known, assume negligible N,P,K content in soil prior to nutrient addition.)
	<ul> <li>a. Nitrogen (N) needed to be added per kg dry soil = line 2b.× 0.15 = mg N/kg soil</li> <li>b. Phosphorus (P) needed to be added per kg dry soil = line 2b.× 0.01 = mg P/kg soil</li> <li>c. Potassium (K) needed to be added per kg dry soil = line 2b.× 0.01 = mg K/kg soil</li> </ul>
5.	Bulk density of soil = kg/m <sup>3,b</sup> (Assume 1,400 kg/m <sup>3</sup> if unknown.)
6.	Nutrients required per m³ of soil:a.kg N/m³ soil = line $4a.\times$ line $5 \div 1,000,000 = kg N/m³$ soilb.kg P/m³ soil = line $4b.\times$ line $5 \div 1,000,000 = kg P/m³$ soilc.kg K/m³ soil = line $4c.\times$ line $5 \div 1,000,000 = kg K/m³$ soil
7.	Pounds of nutrients required per cubic yards of soila.lb N/yd³ soil = line $6a. \times 1.69 = $ lb N/yd³ soilb.lb P/yd³ soil = line $6b. \times 1.69 = $ lb P/yd³ soilc.lb K/yd³ soil = line $6c. \times 1.69 = $ lb K/yd³ soil
8.	Total volume of soil to be treated bybiopile: yd <sup>3</sup>
9.	Pounds of nutrient source to be added per cubic yard of soil: line 7a. ÷ line 1a. = lb of N source required/yd soil line 7b. ÷ line 1b. = lb of P source required/yd soil line 7c. ÷ line 1c. = lb of K source required/yd soil
10.	Total pounds of nutrient sources required for thebiopile: line $9a. \times line 8 =$ lb of N source <sup>(c)</sup> to be purchased line $9b. \times line 8 =$ lb of P source to be purchased line $9c. \times line 8 =$ lb of K source to be purchased
(a)	Weight fraction = $\% \div 100$ .

(a) Weight fraction = 76 + 100.
(b) 1 kg/m<sup>3</sup> = 1.688 lb/yd<sup>4</sup>.
(c) Assumes all N comes from a single source. In this example calculation, it is urea. NA = not applicable.

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Figure J-2. Example of a Completed Biopile Nutrient Addition Worksheet.

	<b>BIOPILE NUTRIENT ADDITION WORKSHEET</b>
1.	Nutrient Source:       a. Nitrogen source (e.g. urea) weight fraction nitrogen (urea = 0.46) <sup>a</sup> b. Phosphorus source (e.g. diammonium phosphate) weight fraction phosphorus         c. Potassium source (e.g. potassium sulfate) weight fraction potassium
2.	Total organic carbon content in soil: mg/kg dry soil. Obtained from laboratory results. If unknown, calculate as below:         a. Average concentration of hydrocarbon contamination in soil = mg/kg dry soil         b. Average carbon content in contamination = line $2a \times 0.8$ = mg carbon/kg dry soil
3.	Desired C:N:P:K ratio. Determine by treatability tests, else use C:N:P:K = 100:15:1:1.
4.	Amount of nutrient to add per kg of dry soil. (If not known, assume negligible N,P,K content in soil prior to nutrien addition.)
	<ul> <li>a. Nitrogen (N) needed to be added per kg dry soil = line 2b.× 0.15 = mg N/kg soil</li> <li>b. Phosphorus (P) needed to be added per kg dry soil = line 2b.× 0.01 = mg P/kg soil</li> <li>c. Potassium (K) needed to be added per kg dry soil = line 2b.× 0.01 = mg K/kg soil</li> </ul>
5.	Bulk density of soil = kg/m <sup>3,b</sup> (Assume 1,400 kg/m <sup>3</sup> if unknown.)
6.	Nutrients required per m <sup>3</sup> of soil: a. $kg N/m^3 soil = line 4a. \times line 5 \div 1,000,000 = kg N/m^3 soil$ b. $kg P/m^3 soil = line 4b. \times line 5 \div 1,000,000 = kg P/m^3 soil$ c. $kg K/m^3 soil = line 4c. \times line 5 \div 1,000,000 = kg K/m^3 soil$
7.	Pounds of nutrients required per cubic yards of soila.lb N/yd³ soil = line $6a. \times 1.69 = $ lb N/yd³ soilb.lb P/yd³ soil = line $6b. \times 1.69 = $ lb P/yd³ soilc.lb K/yd³ soil = line $6c. \times 1.69 = $ lb K/yd³ soil
8.	Total volume of soil to be treated bybiopile: yd <sup>3</sup>
9.	Pounds of nutrient source to be added per cubic yard of soil: line 7a. ÷ line 1a. = lb of N source required/yd soil line 7b. ÷ line 1b. = lb of P source required/yd soil line 7c. ÷ line 1c. = lb of K source required/yd soil
10.	Total pounds of nutrient sources required for thebiopile: line $9a. \times line 8 =$ lb of N source <sup>(c)</sup> to be purchased line $9b. \times line 8 =$ lb of P source to be purchased line $9c. \times line 8 =$ lb of K source to be purchased
(a)	Weight fraction = $\% \div 100$ .

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Figure J-3. Biopile Nutrient Addition Worksheet.

<sup>(</sup>b) 1 kg/m<sup>3</sup> = 1.688 lb/yd<sup>4</sup>.
(c) Assumes all N comes from a single source. NA = not applicable.

#### lb N-source needed :

First calculate amount of N supplied by DAP  $(NH_3)_2 \text{ HPO}_4 \text{ is } 21.5\% \text{ N} = 0.22 \text{ lb } \text{N/lb } \text{DAP}$ N supplied by DAP = (800 lb DAP) × 0.22 lb N/1 lb DAP) = 176 lb N from DAP Primary N-source = urea 1 lb N/lb urea = 0.46 Urea needed = (2.880 lb N - 176 lb N from DAP) ÷ (0.46 lb N/lb urea)

```
Urea needed = (2,880 \text{ lb N} - 176 \text{ lb N from DAP}) \div (0.46 \text{ lb N/lb urea}
= 5,880 lb urea
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Nutrients to purchase : 5,880 lb urea; 800 lb diammonium phosphate

#### K-source needed:

Addition of potassium (K) usually is not required. Where treatability studies indicate insufficient K, the K can be added in half the amount of P. Potash ( $K_{CO_3}$ ) is a common K-source.

**J.4** Calculating the Nutrient Application Rate . To ensure even nutrient distribution, N- and P-sources should be applied uniformly throughout the soil. Therefore, the nutrient application ratelb of each nutrient added per unit mass or volume of soil) should be calculated.

From earlier calculations, it is known that 2,400 mg N/kg soil and 160 mg P/kg soil will be needed.

Amount DAP required/kg soil = (160 mg P/kg soil) (1 mg DAP/0.24 mg P) = 667 mg DAP/kg soil = 0.667 kg DAP/1,000 kg soil

Amount urea required/kg soil = (2,400 - 0.22(667)) (1/0.46) = 4,898 mg urea/kg soil = 4.90 kg urea/1,000 kg soil

Assuming a soil density of 1,420 kg/m<sup>3</sup>, the amount of nutrients needed per unit volume can be calculated: (0.667 kg DAP/1,000 kg soil) (1,420 kg/m<sup>3</sup>) = **0.95 kg DAP/m**<sup>3</sup> soil

 $(4.90 \text{ kg urea}/1,000 \text{ kg soil}) (1,420 \text{ kg/n}) = 6.96 \text{ kg urea/m}^3 \text{ soil}$ 

Converting kg/m³ to lb/yd3:

 $\begin{array}{ll} (0.95 \text{ kg DAP/m}^3 \text{ soil}) & (2.205 \text{ lb/kg}) & (0.765 \text{ m}^3/\text{yd}^3) &= 1.6 \text{ lb DAP/yd}^3 \text{ soil} \\ (6.96 \text{ kg urea/m}^3 \text{ soil}) & (2.205 \text{ lb/kg}) & (0.765 \text{ m}^3/\text{yd}^3) &= 11.7 \text{ lb urea/yd}^3 \text{ soil} \end{array}$ 

 $J.5 \qquad \mbox{Calculating the Initial Moisture Addition Requirement} \ . \ From the design scenario, the soil contains 10% H_2O and is at 50% of field capacity. The target field capacity will be 95%.$ 

% moisture at which soil will be at 95% field capacity:

Make the simplifying assumption that there is a linear relationship between moisture content and field capacity.

50% field capacity/10% moisture = 95% field capacity  $\div$  x% moisture

x% moisture = (95%)  $(10\%) \div 50\% = 19\%$ 

Therefore, enough water must be added to the soil to bring the moisture content to 19%.

#### Amount of water to be added per kg of soil :

Prior to moisture addition, 1 kg soil contains 0.1 kg HO. After moisture addition, 1 kg soil should contain 0.19 kg HO.

In 1 kg of soil at 19% moisture, 0.81 kg would be dry soil. The soil on hand contains 10% moisture.

At 10% moisture, the total mass of soil containing 0.81 kg dry soil would be (0.81/0.9) = 0.9 kg.

The 0.9 kg of 10% H<sub>2</sub>O soil would therefore contain 0.81 kg dry soil; and 0.09 kg H<sub>2</sub>O.

To get to 1 kg of soil at 19% moisture, 0.1 kg HO must be added to every 0.9 kg of 10% HO soil, which

equals: